

# Tank and Support Structure Design and Analysis

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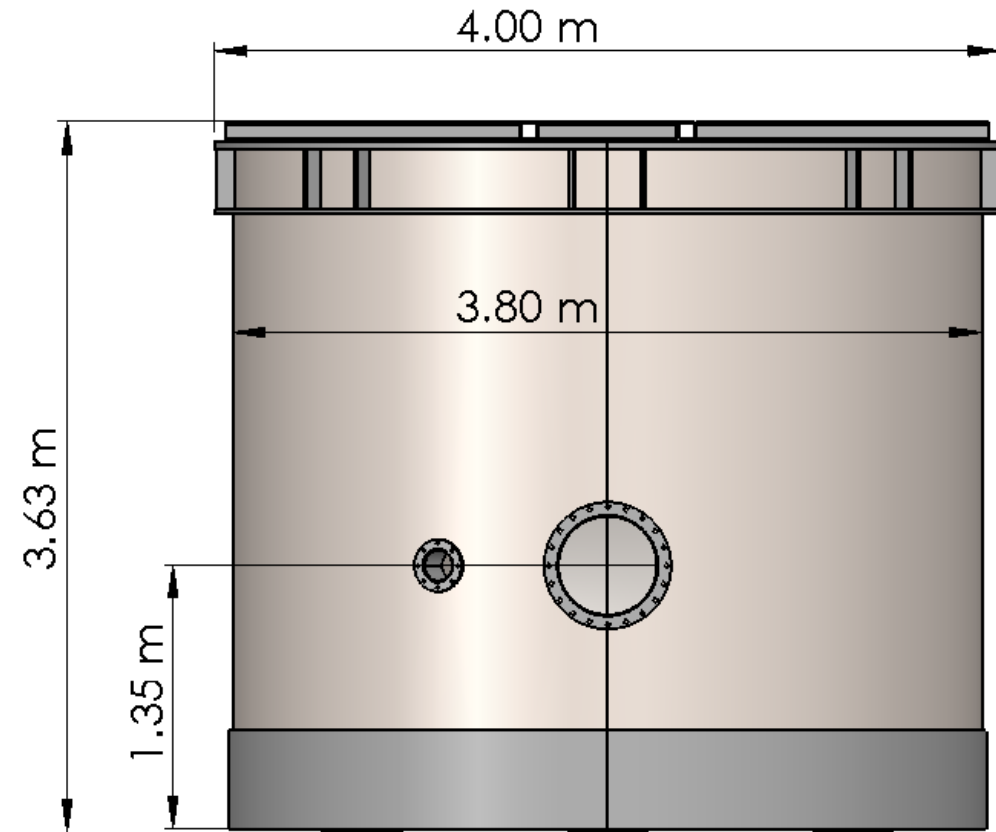
# Design Overview

MATERIAL PROPERTIES		
Material	Stainless steel	
Grade	SS 304	
Poisson's ratio	0.31	
Density	7750	kg/m <sup>3</sup>
Modules of Elasticity	193 X 10 <sup>3</sup>	M Pa
<b>Design Yield Strength</b>	<b>207</b>	<b>M Pa</b>
Ultimate Tensile Strength	586	M Pa
Thermal Coefficient	1.7 X 10 <sup>-5</sup>	/°C

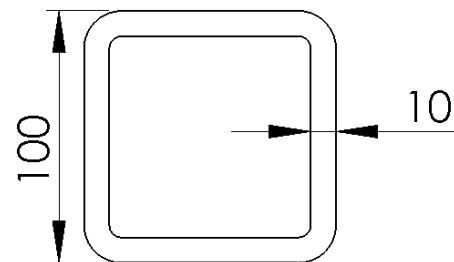
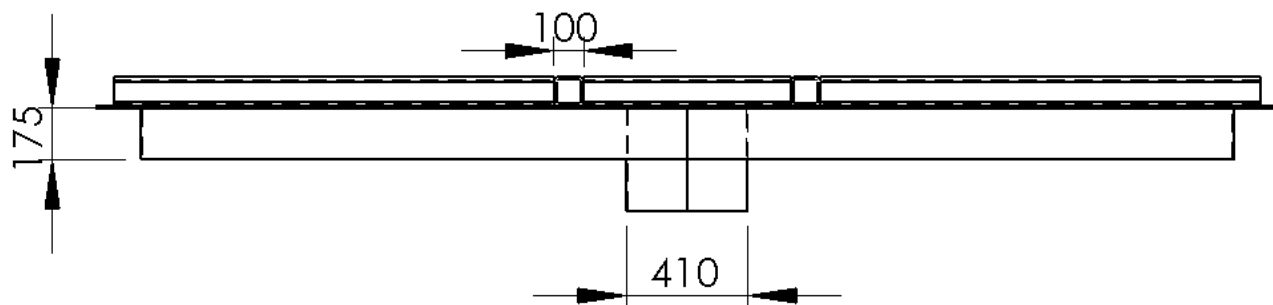
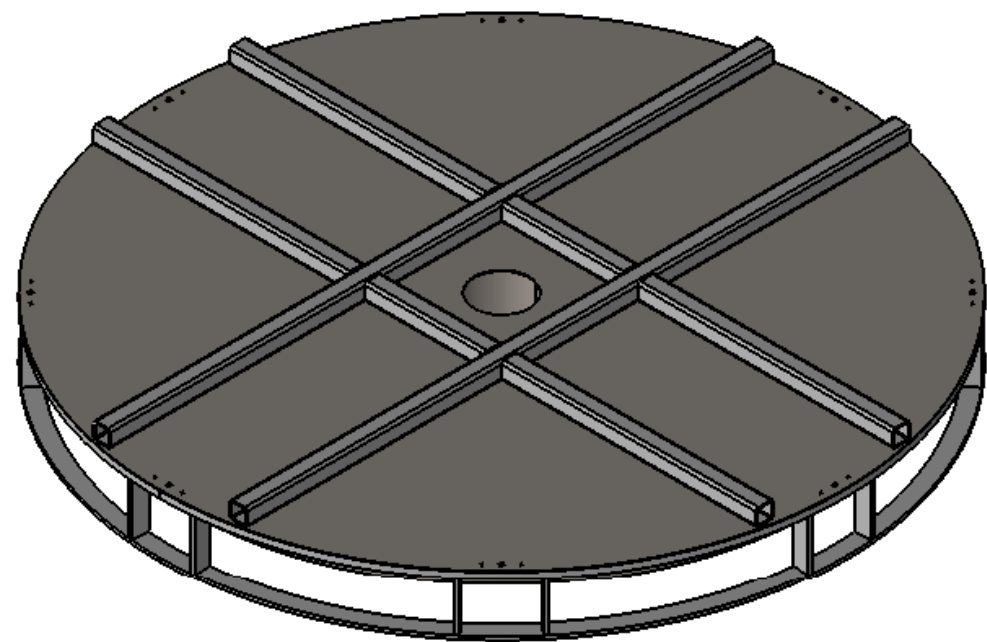
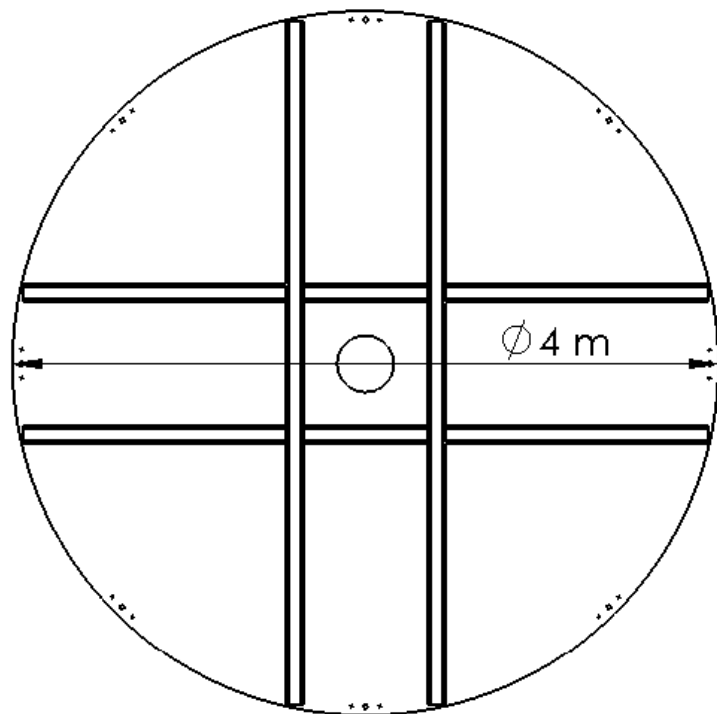


# Design Overview

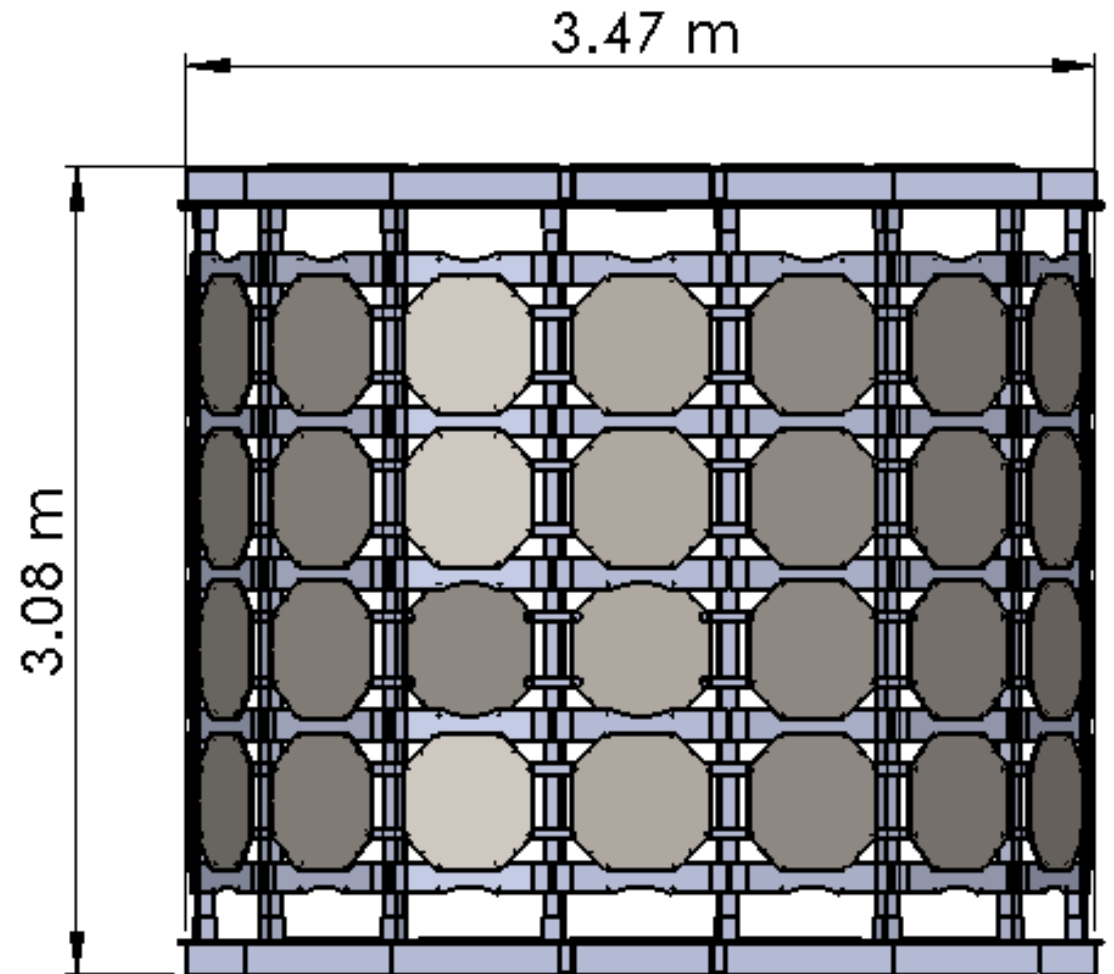
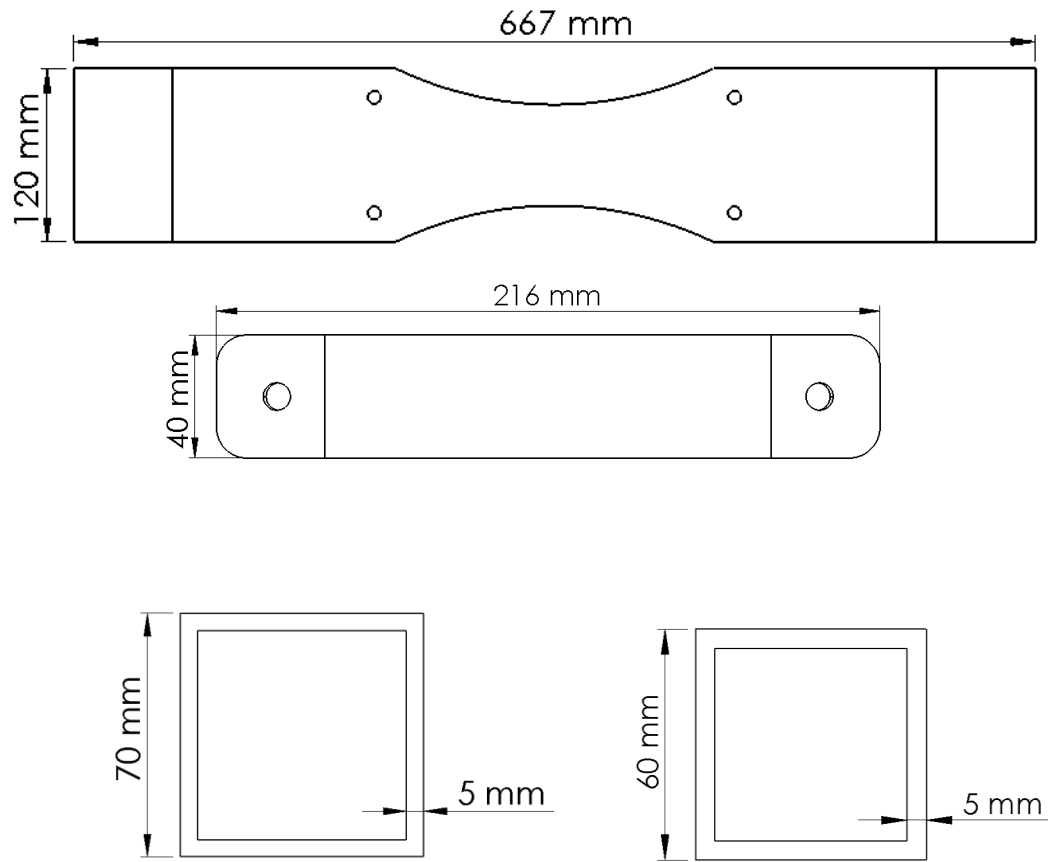
COMPONENT	MASS ( TON)
Tank	3.2
Base	2.2
Lid	1.0
Water	38.5
Support Structure +mPMTs	6.0
CDS	0.1
Miscellaneous*	0.2
<b>Total</b>	<b>~51 TON</b>



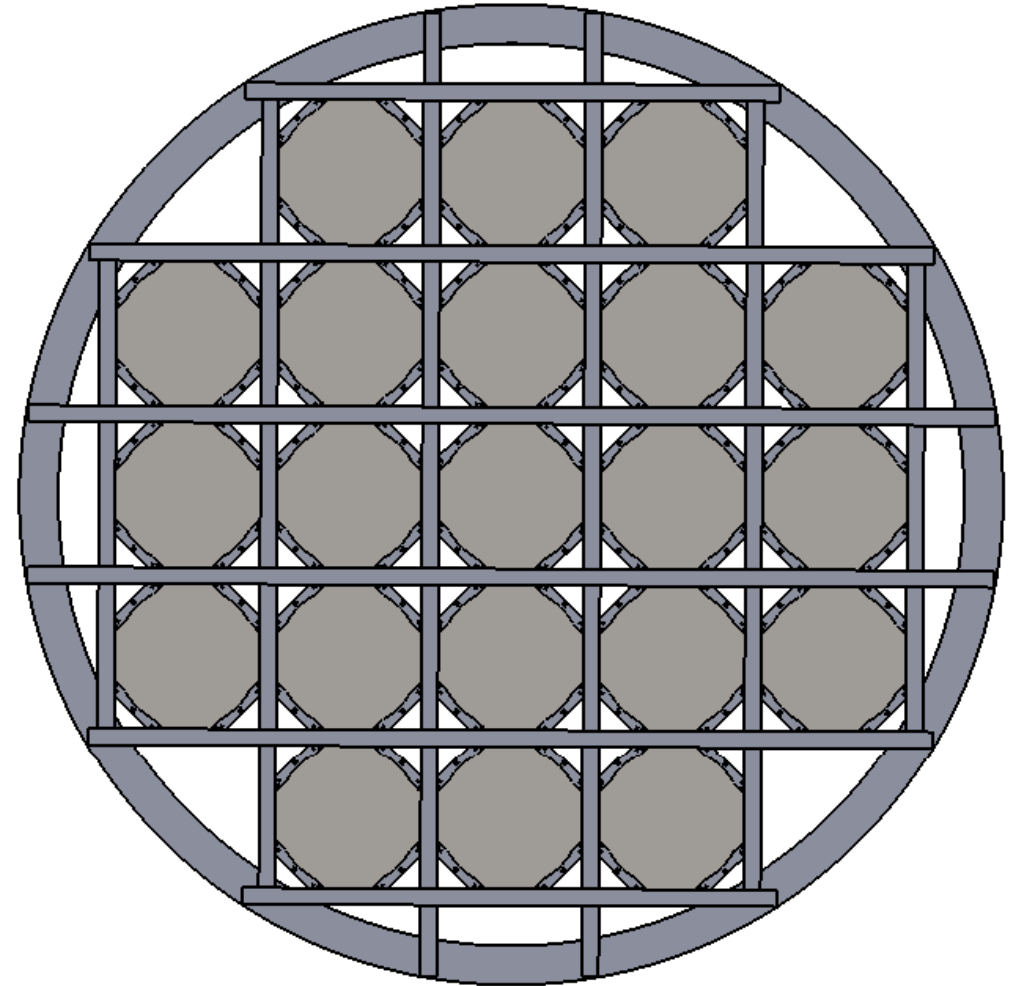
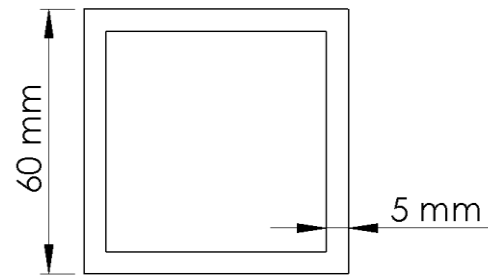
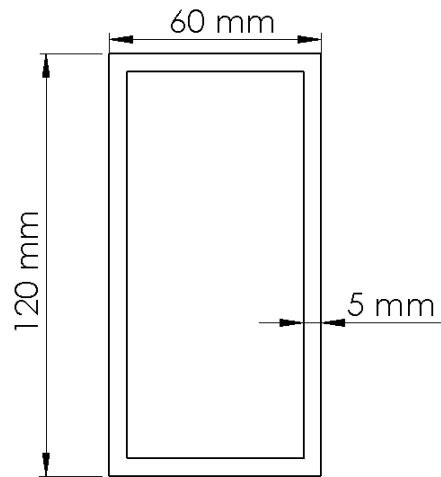
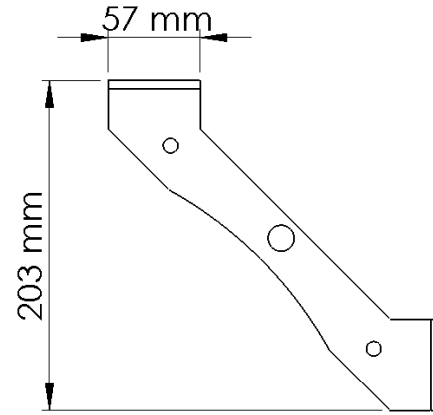
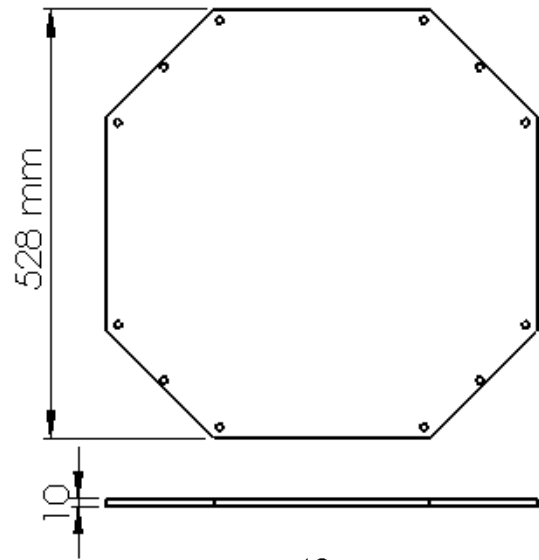
# Lid Details



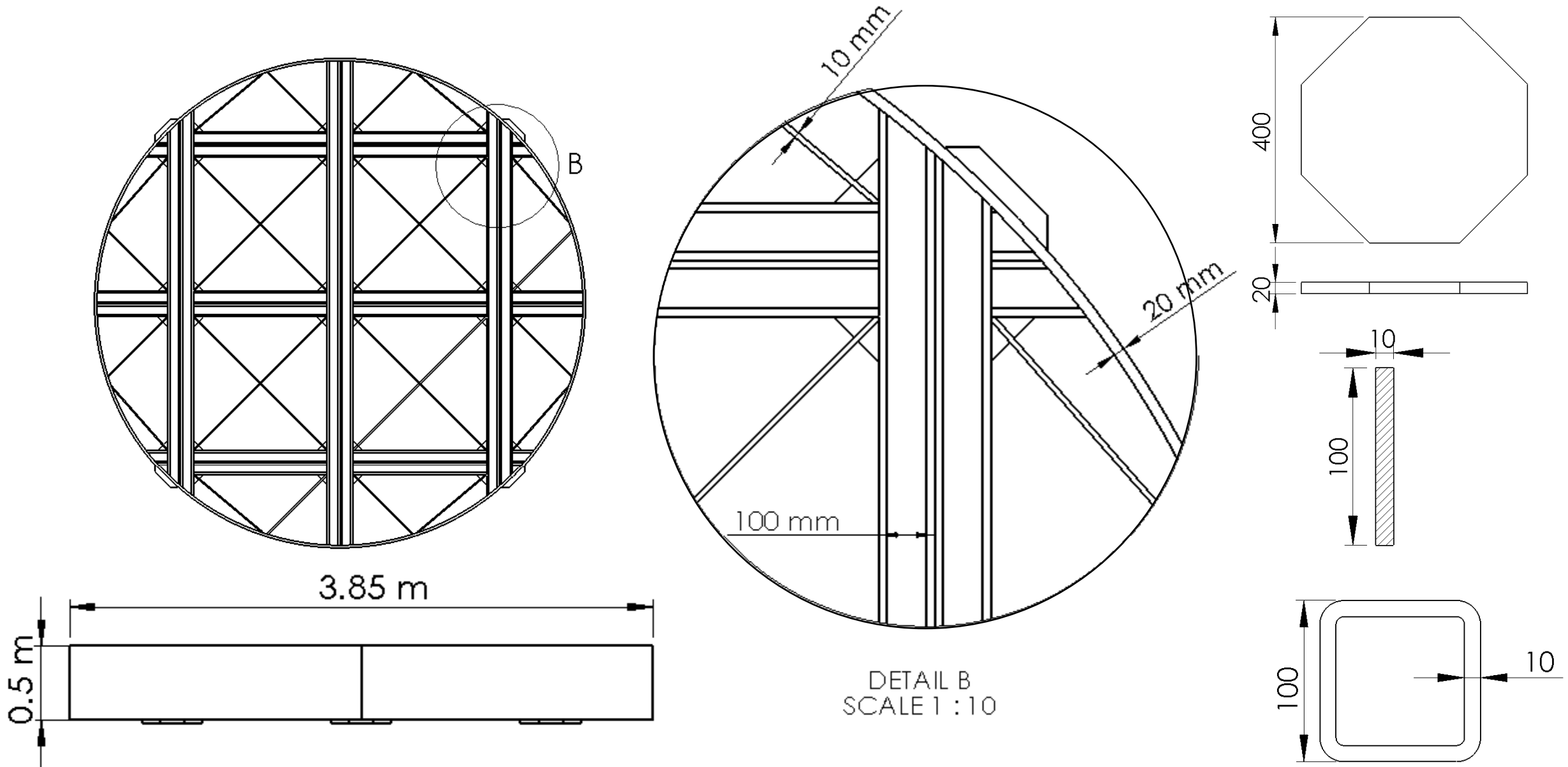
# Support Structure Details



# End Cap Details



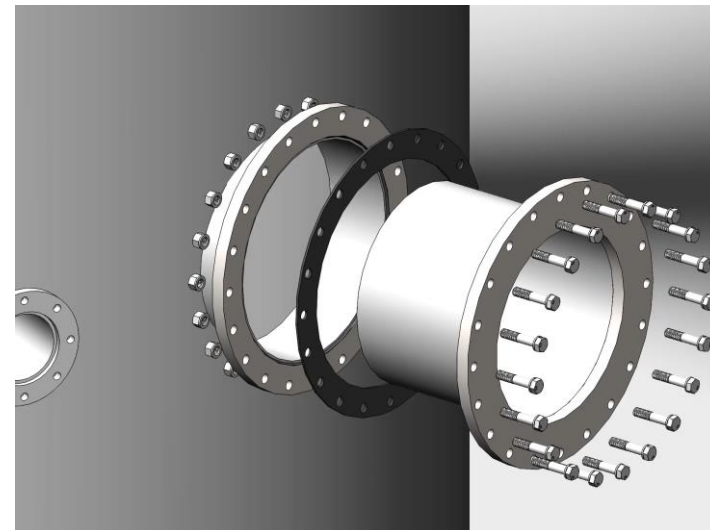
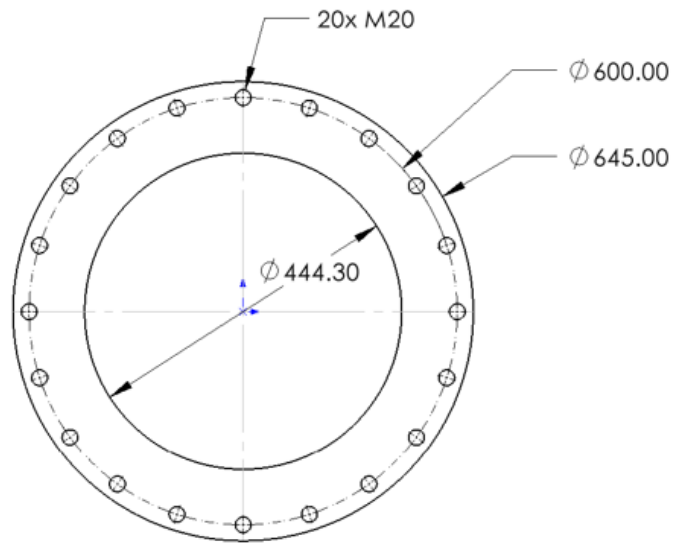
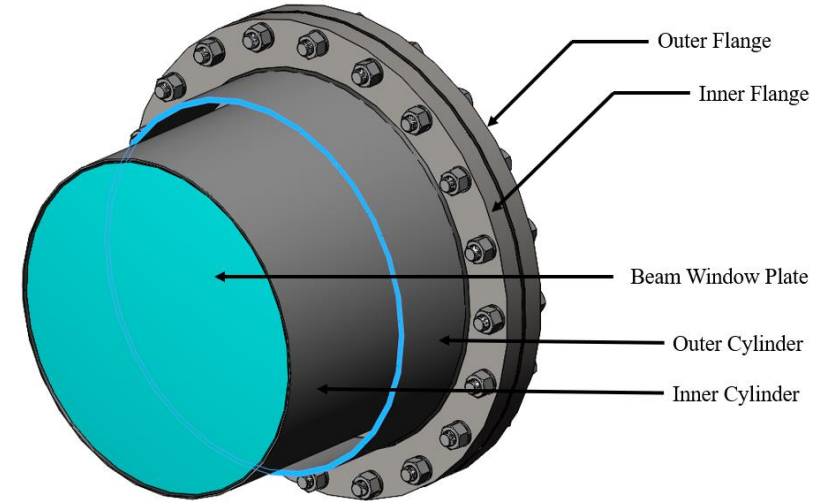
# Base Details





# Tertiary Beam Window Details

Part	Standard	Size
Inner Cylinder	Schedule 10	18-inch
Outer Cylinder	Schedule 5s	20-inch
Inner Flange	EN 1092-1	DN 500
Outer Flange	Custom made	
Flange Bolts, Nuts & Washer		M20



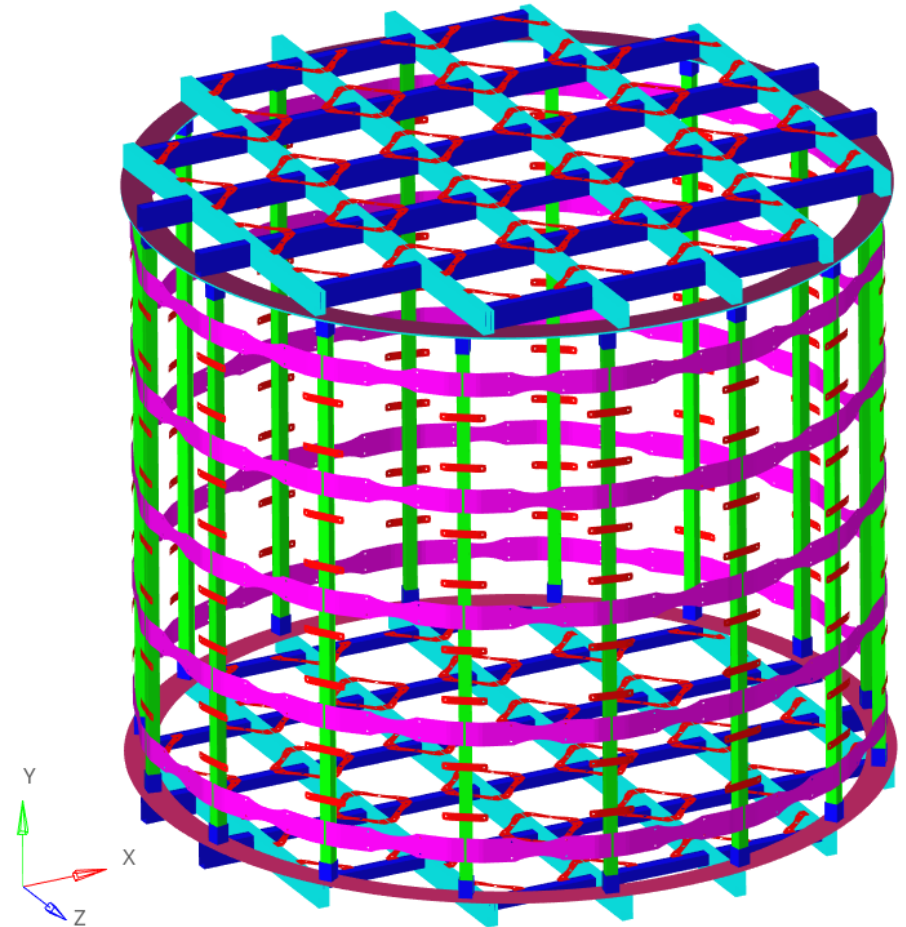
# Support Structure Static Analysis

## Boundary Condition

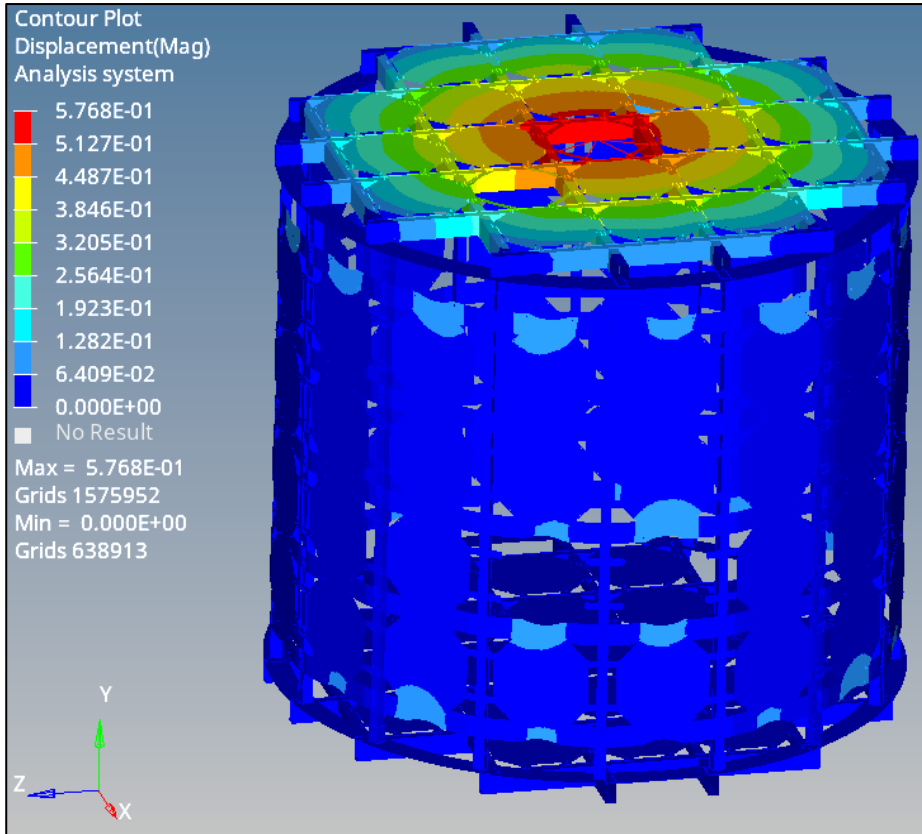
The bottom beams are fixed to all dofs

## Loads

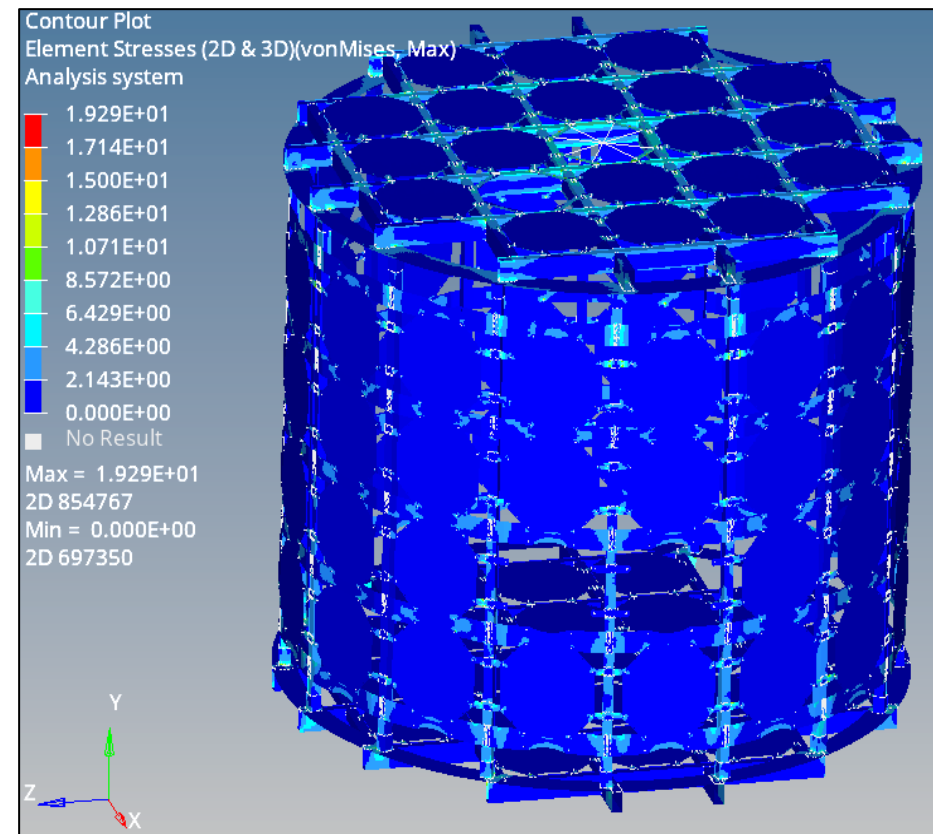
Self wt.( Gravity)+ 102 mPMT wt. 40kgs each



# Support Structure Static Analysis



**Max. Displacement = 0.58 mm**



**Max. Stress = 19.30 MPa**

# Conclusion

1. From the static analysis performed on the two configurations of the support structure, we can infer that the **maximum stress generated is around 20 MPa**.
2. **The FOS is 10**, thus we can conclude that the structure is safe under given loading conditions.

Sr. No.	Top End cap configuration	Displacement (mm) Maximum	vonMises stress (MPa) Maximum	F.O.S
1.	Diagonal mPMT removal	0.58	19.29	10.73

# Lid Static Analysis

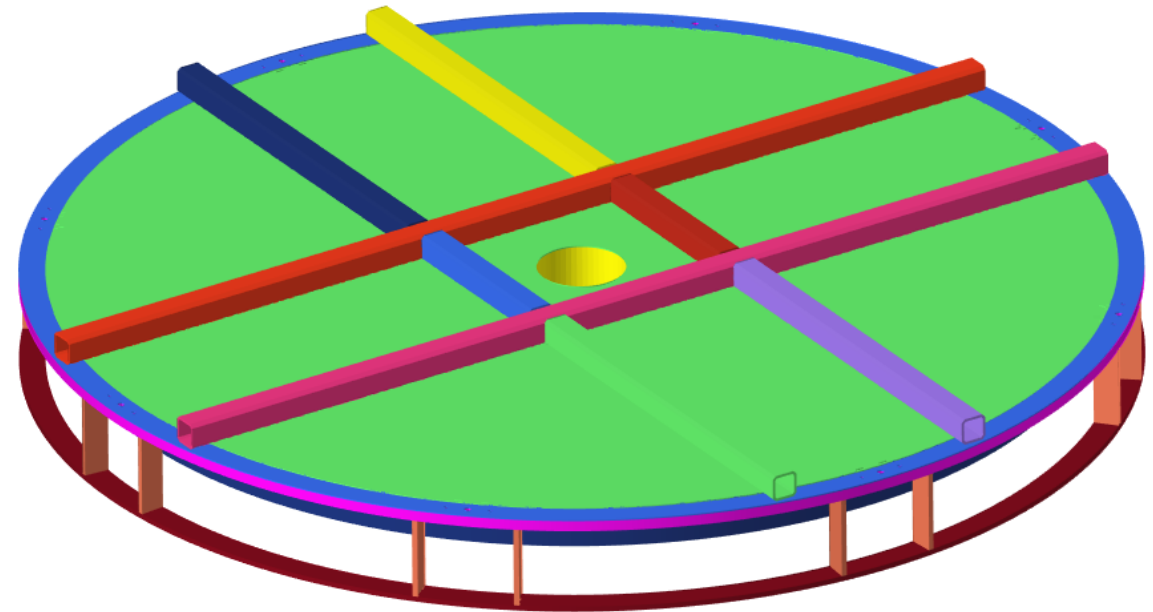
## Boundary Condition

Contact between the lid and tank edge is defines.

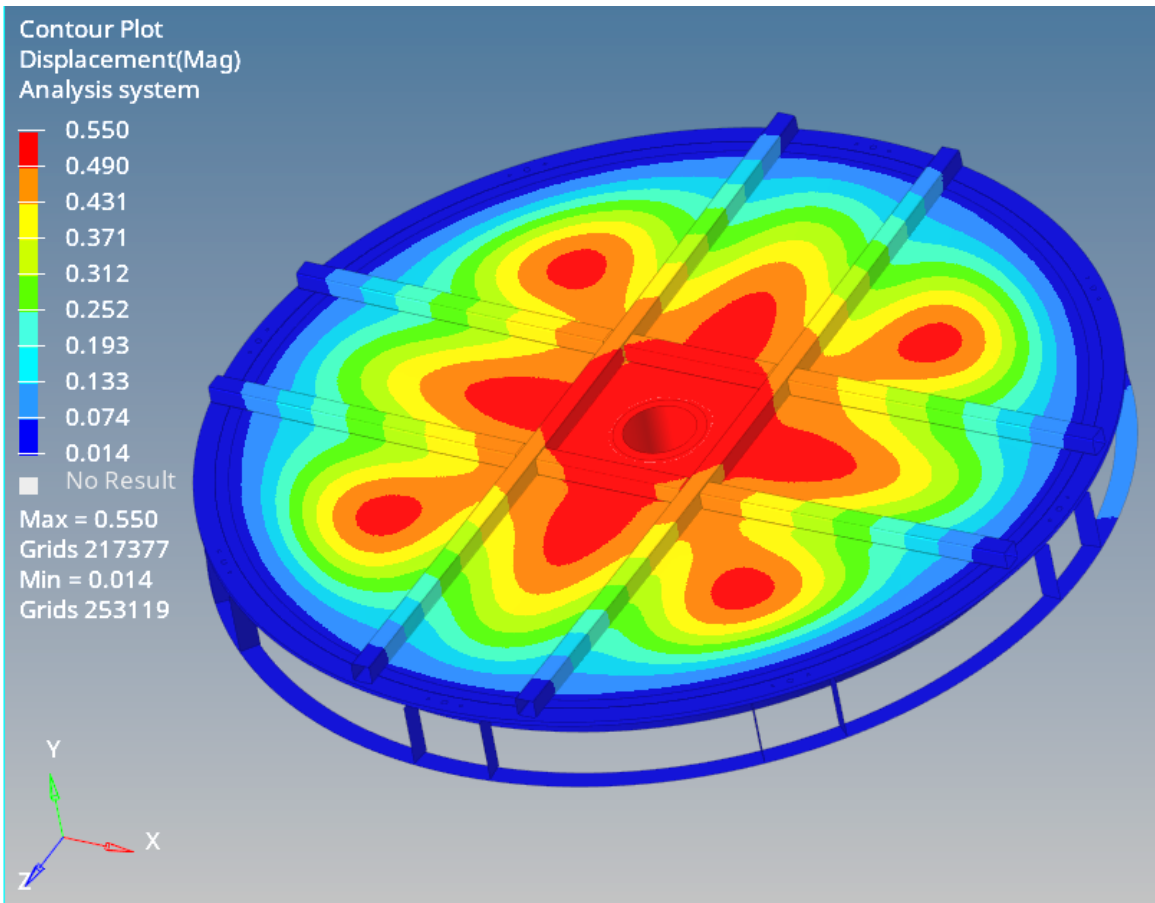
The outer rings and ring connecting members are in contact with the tank surface

## Loads

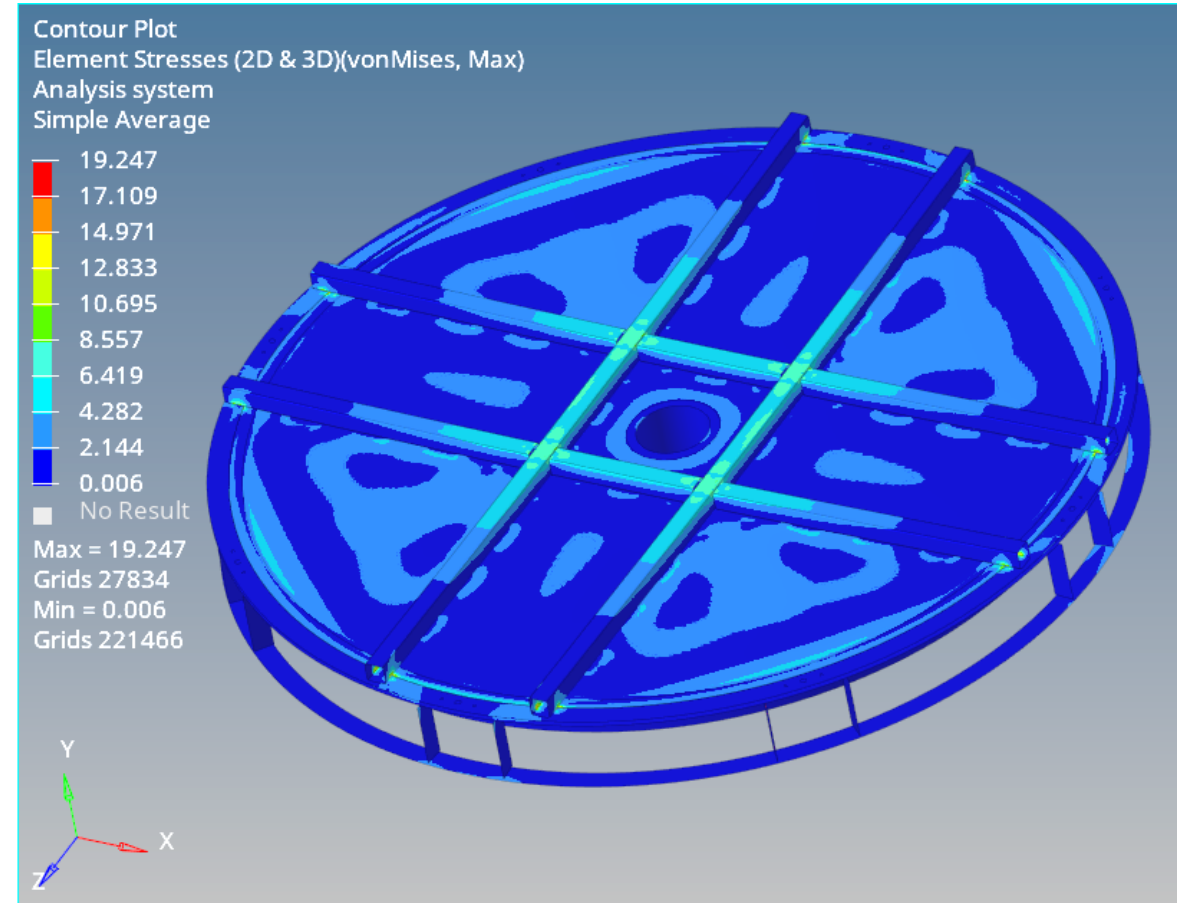
Self wt.( Gravity)+ 100 kg (CDS) on top rafters



# Lid Results

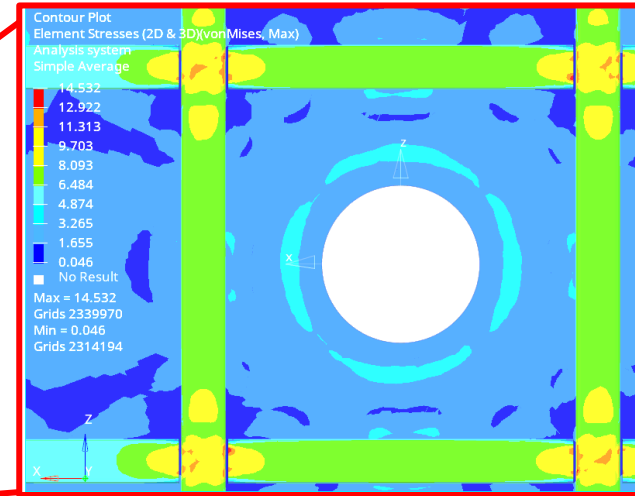
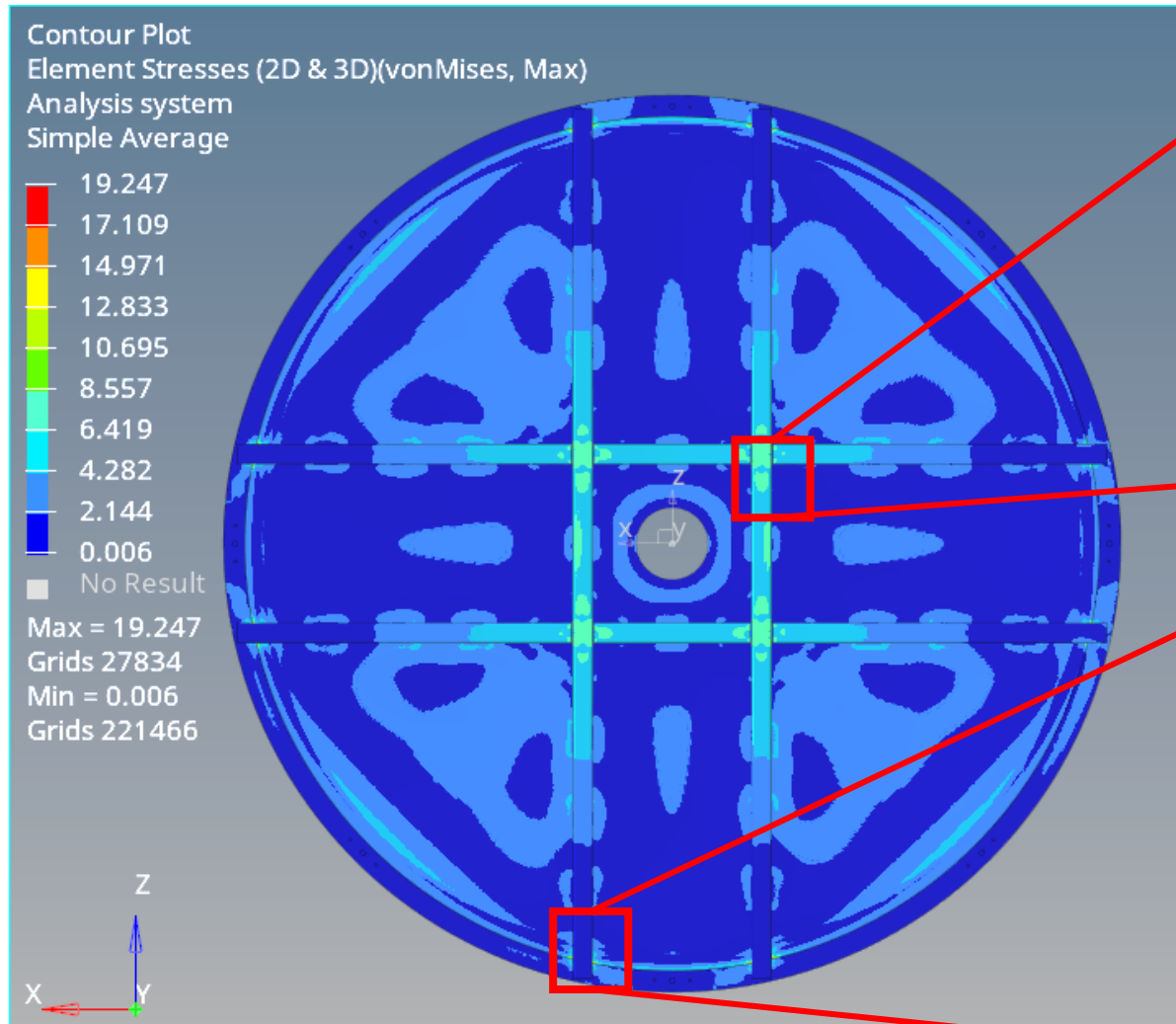


**Max. Displacement = 0.55 mm**

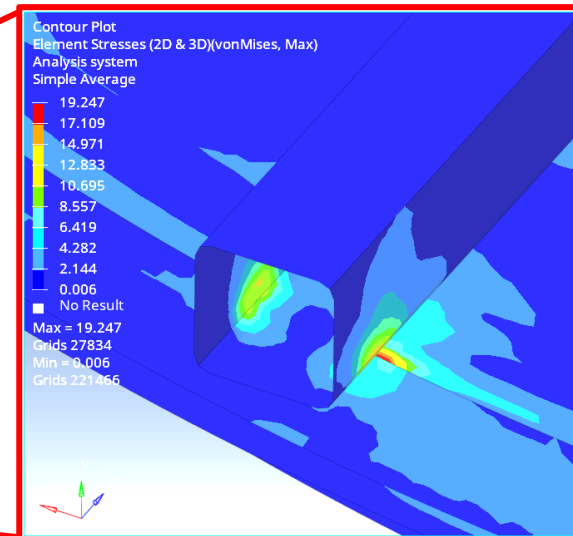


**Max. Stress = 19.24 MPa**

# Lid Results



**14.24 MPa**

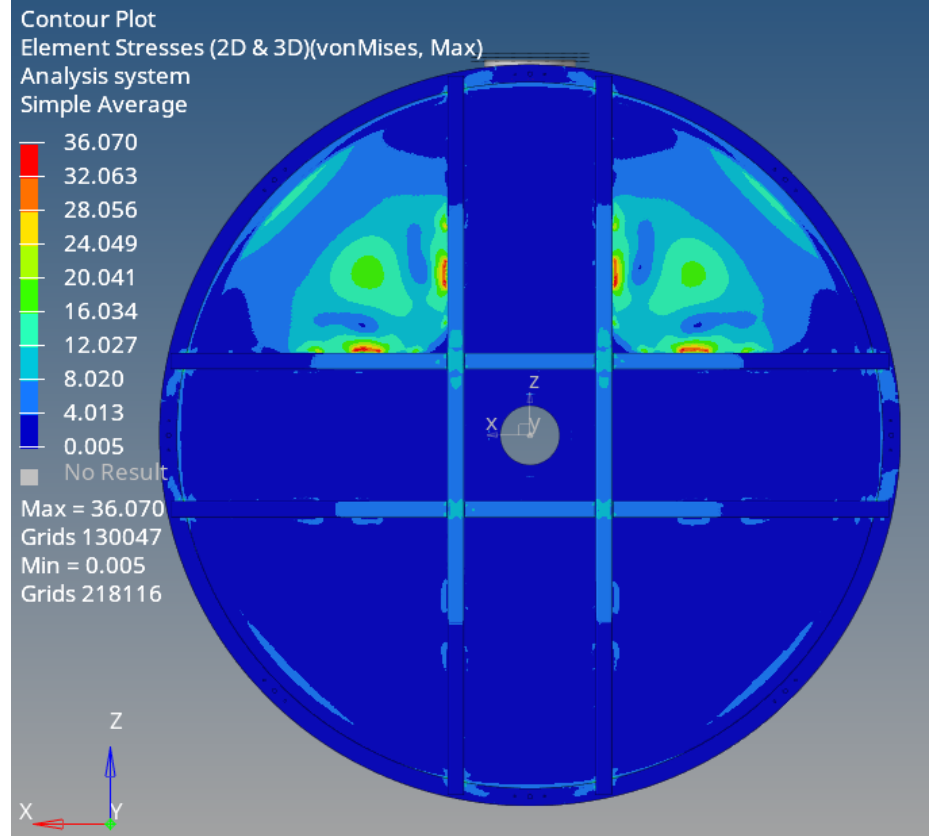
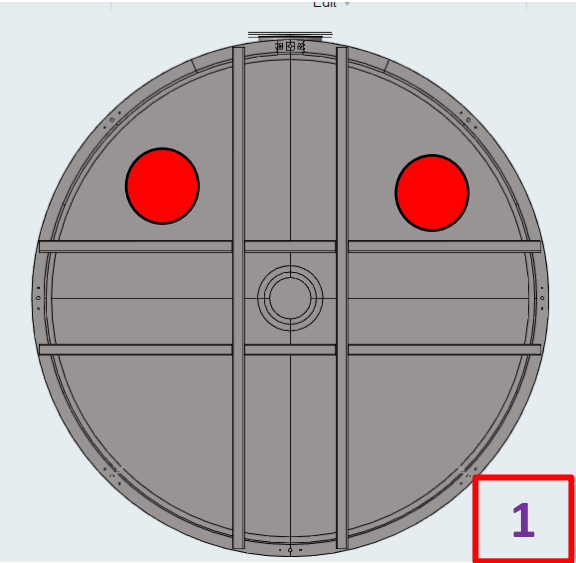


**19.24 MPa**

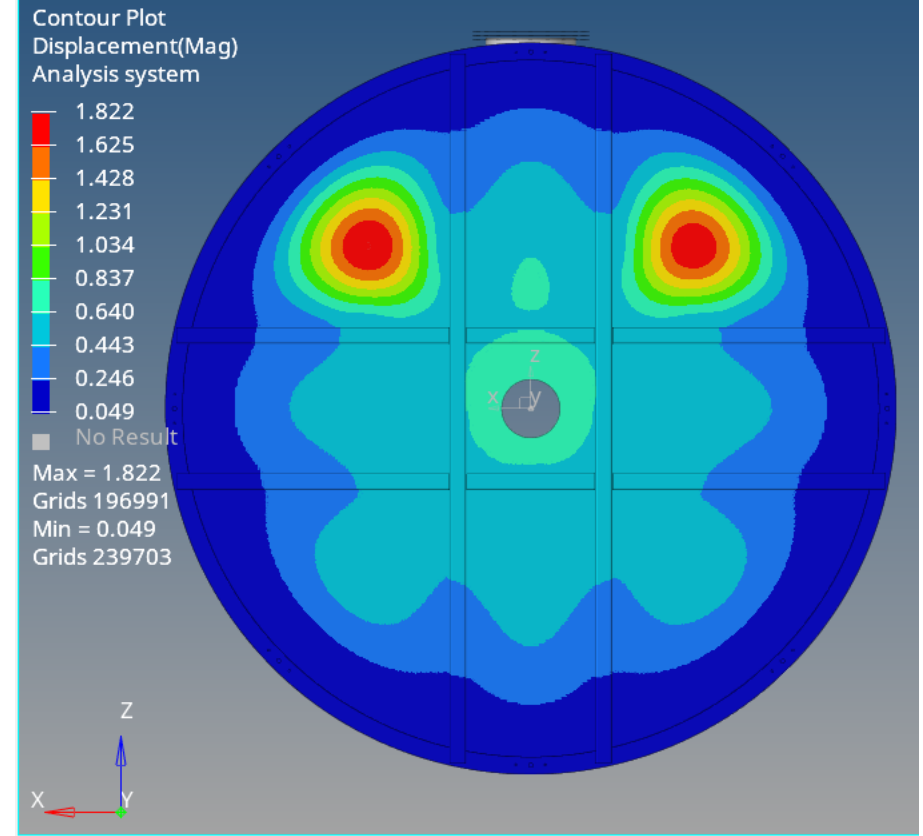


↑  
Beam Window

**Case 1**



**Lid Stress = 36.070 MPa**

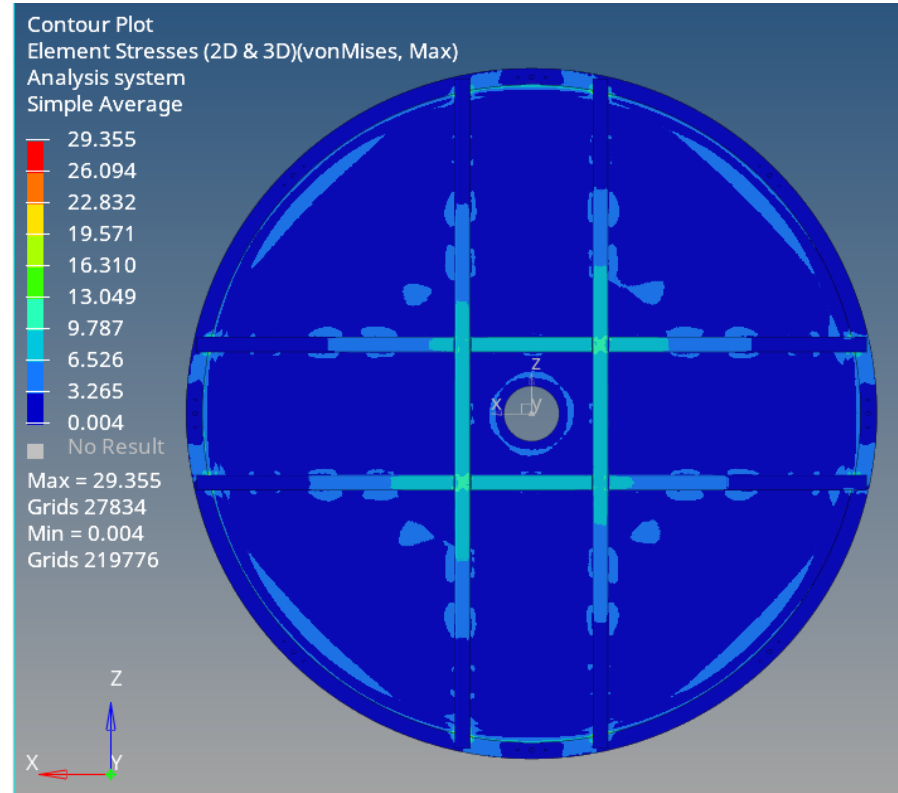
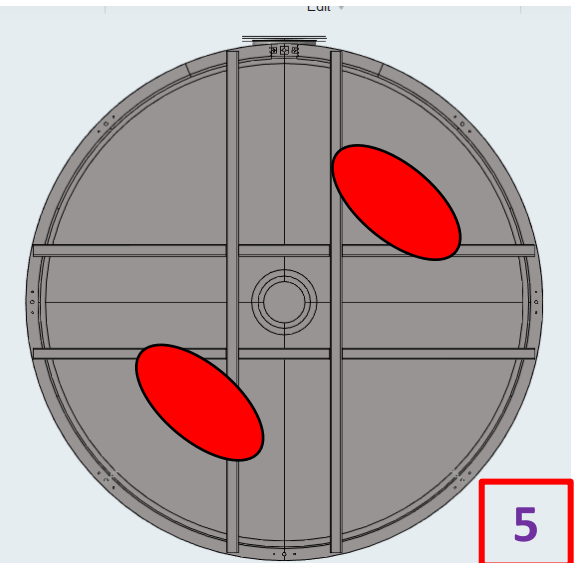


**Lid Displacement = 1.822 mm**

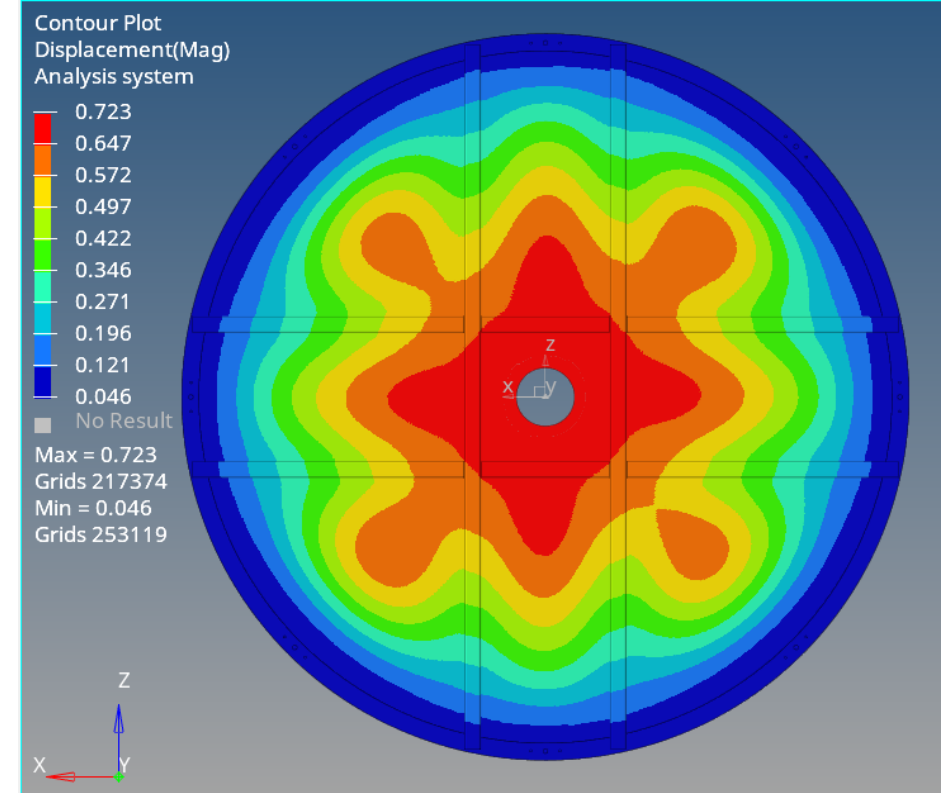


↑  
Beam Window

**Case 5**



**Lid Stress = 29.355 MPa**



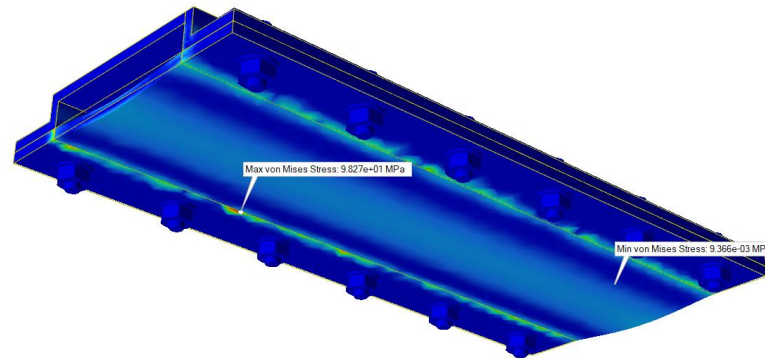
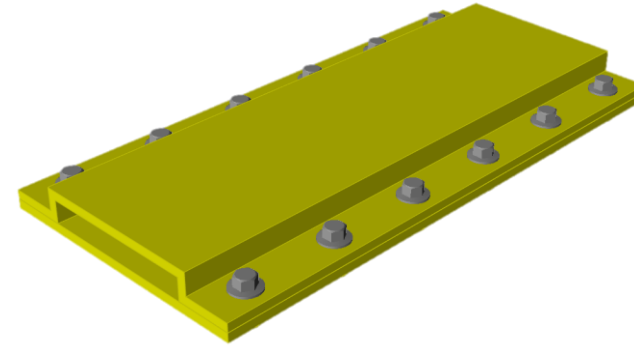
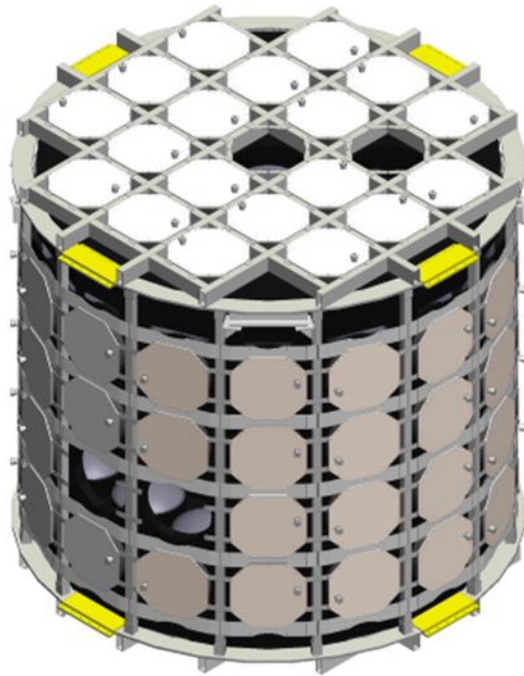
**Lid Displacement = 0.723 mm**

## Conclusion

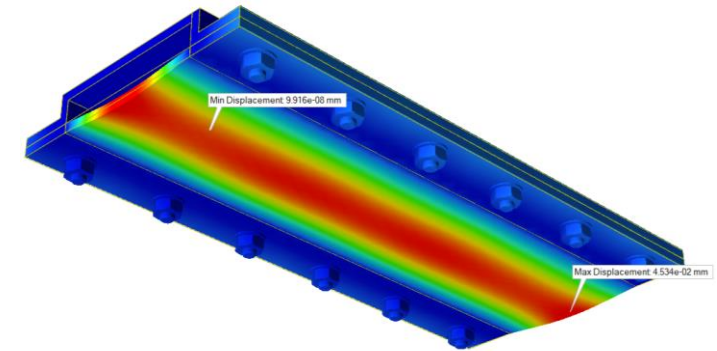
- Under expected loading conditions the maximum stress on the lid is ~ **20 MPa** with a displacement of **0.55 mm**.
- From the Tank Lid Safety studies, we can infer that direct load on lid ( Case 1) causes higher stress compared to other cases.
- The minimum factor of safety is **5.7** for the lid.

Lid Case Study			
Case	Max. VonMises stress (MPa)	Max. Displacement (mm)	FOS
1	36.070	1.822	5.7
5	29.355	0.723	7.0

# Support Ring Clamps Static Analysis



**Max. Stress = 98 MPa**



**Max. Displacement = 0.05 mm**

# Hydrostatic Analysis

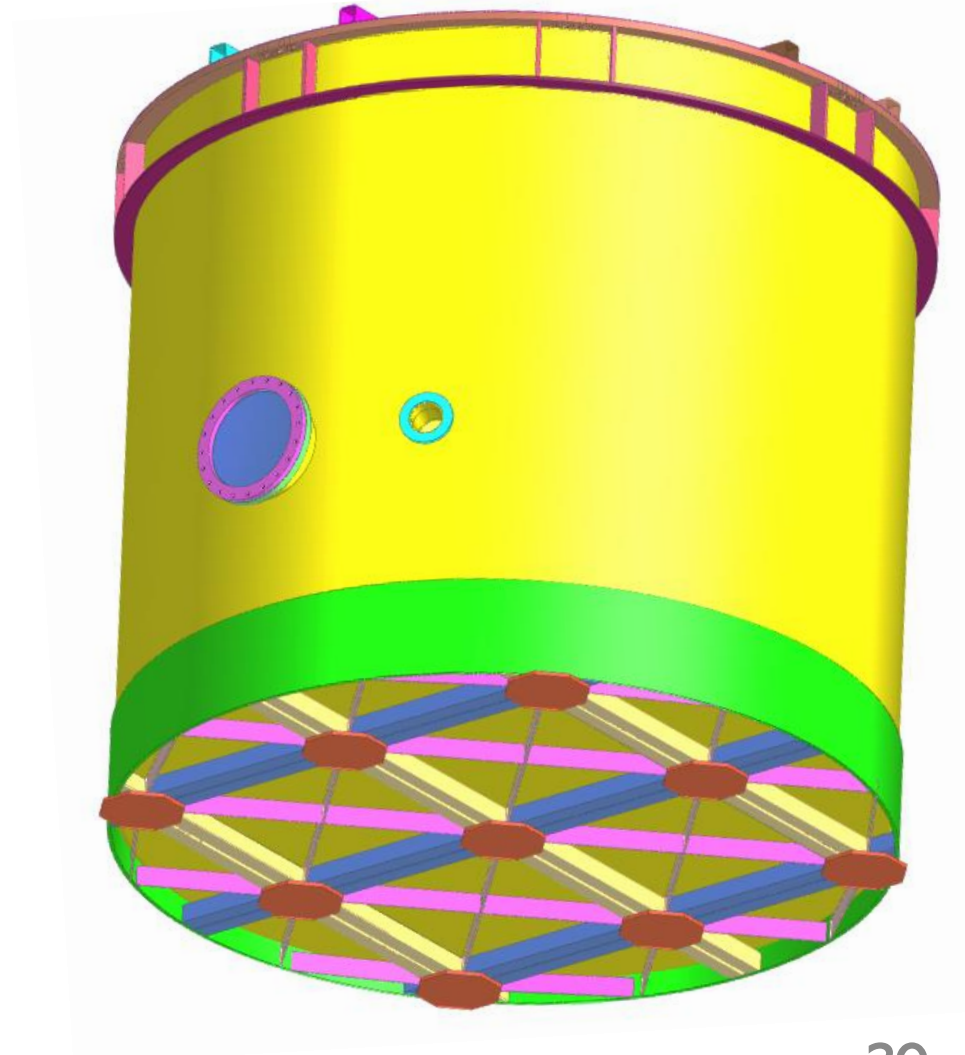
## Boundary Condition

The bottom plates are restricted to all dofs.

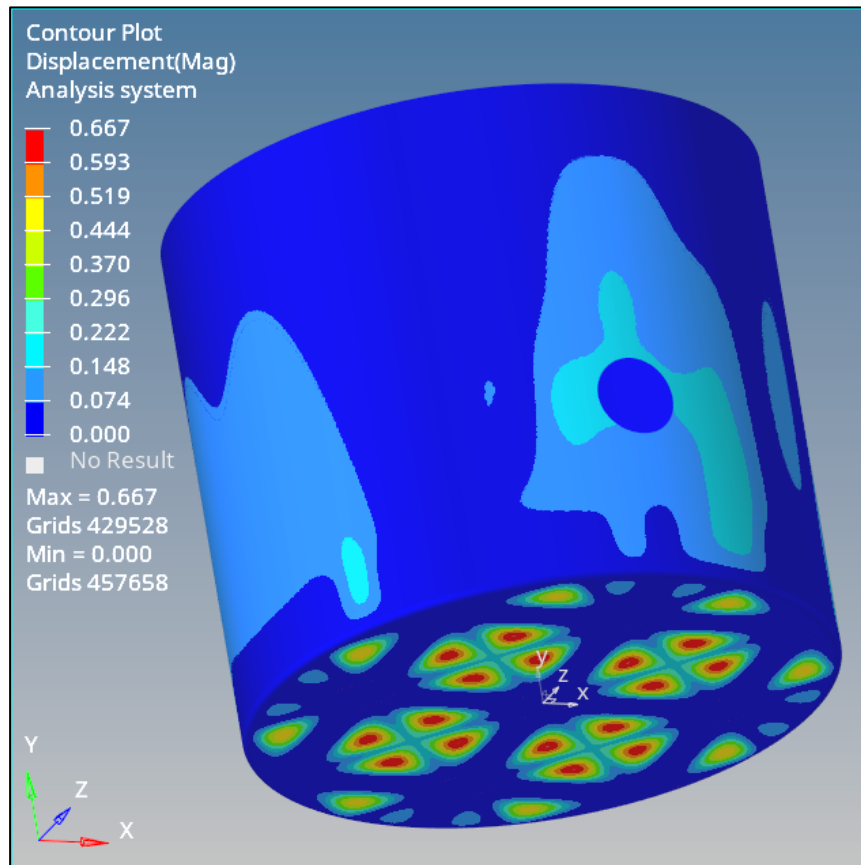
## Loads

Self wt.( Gravity)+ Hydrostatic Pressure on tank walls  
( ht. of water =3.4m) +500 kg (CDS) on top rafters

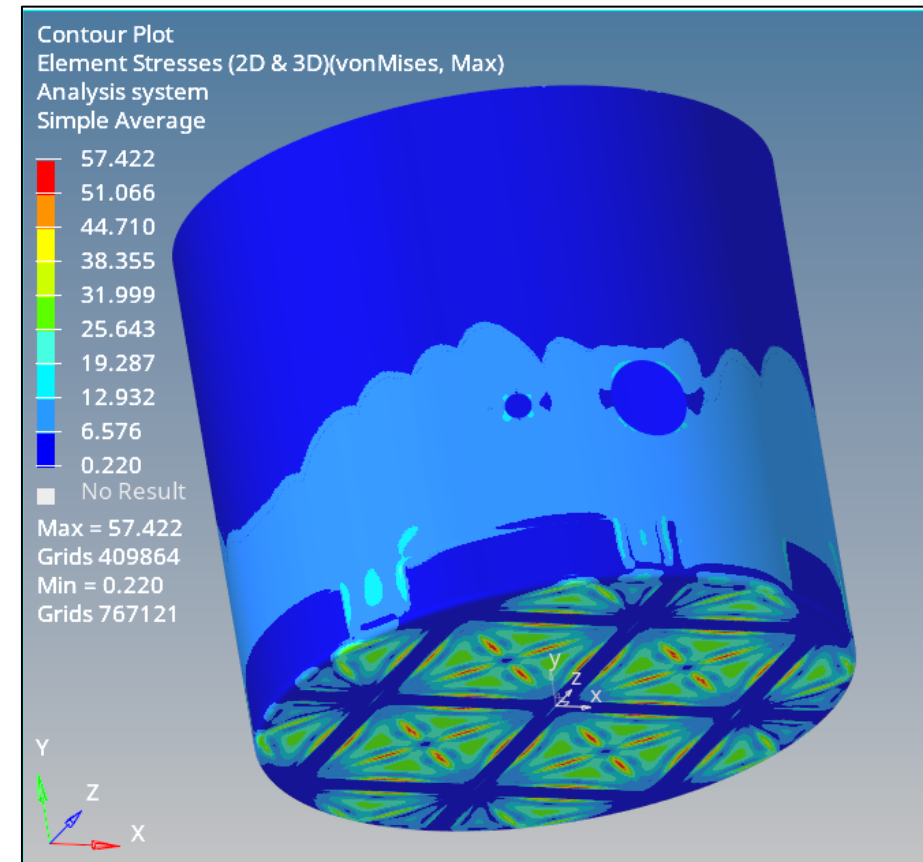
- Wt. of Water = 38.50 tons
- Ht. of Water level = 3400mm
- Tank Wall Thickness = 6mm



# Tank Results

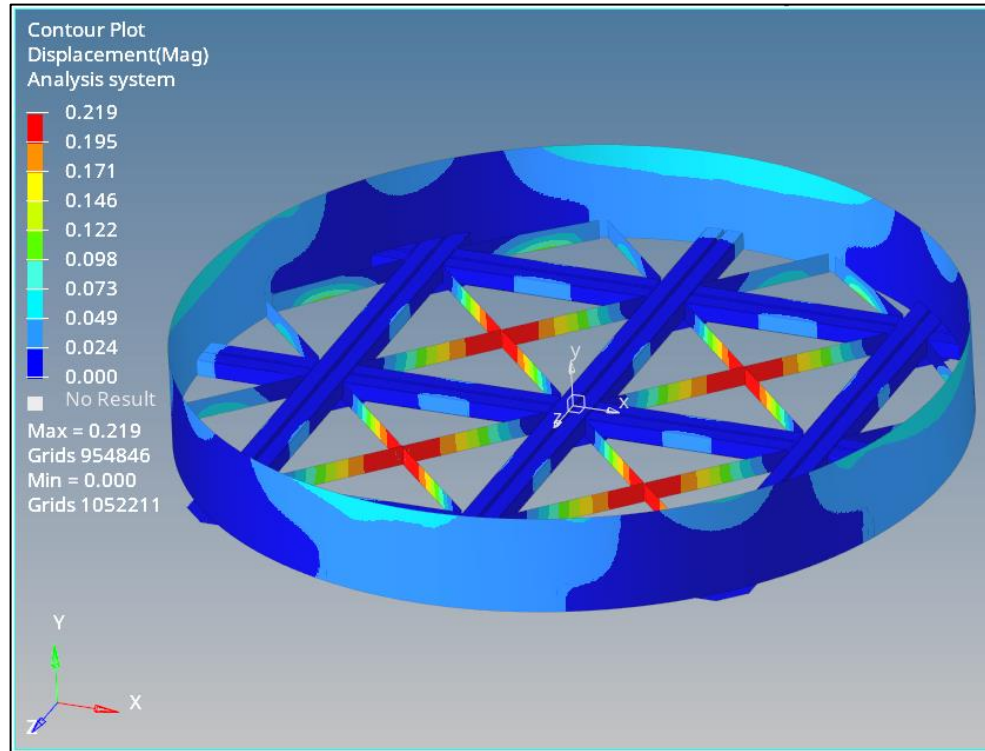


**Max. Displacement = 0.67 mm**

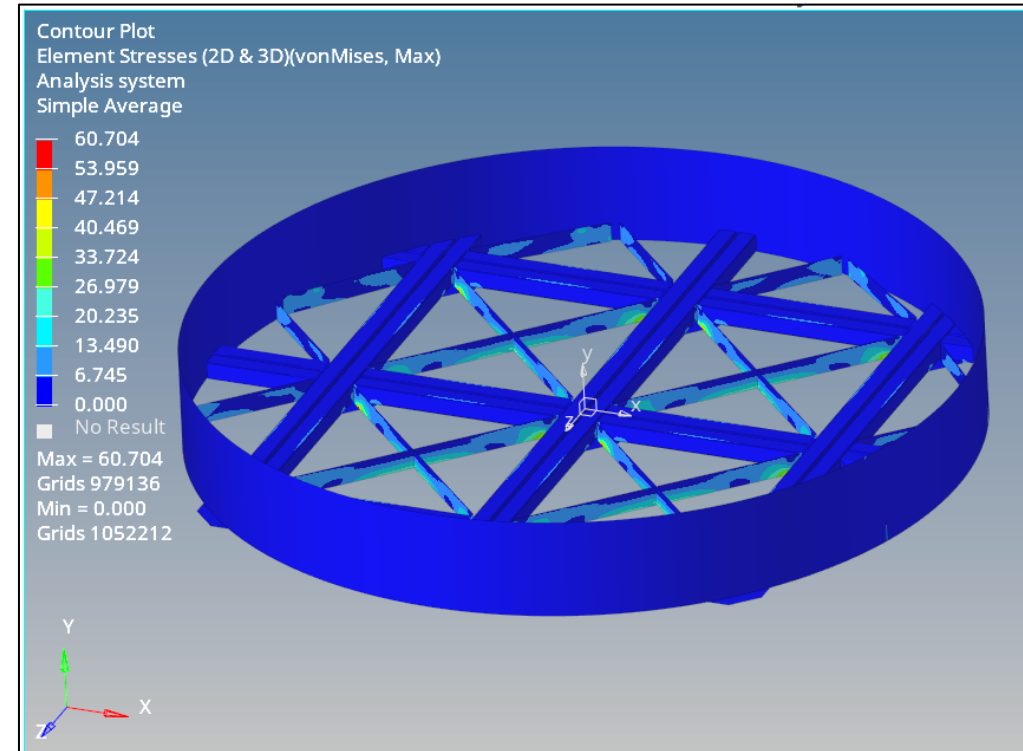


**Max. Stress = 57.42 MPa**

# Base Support Results



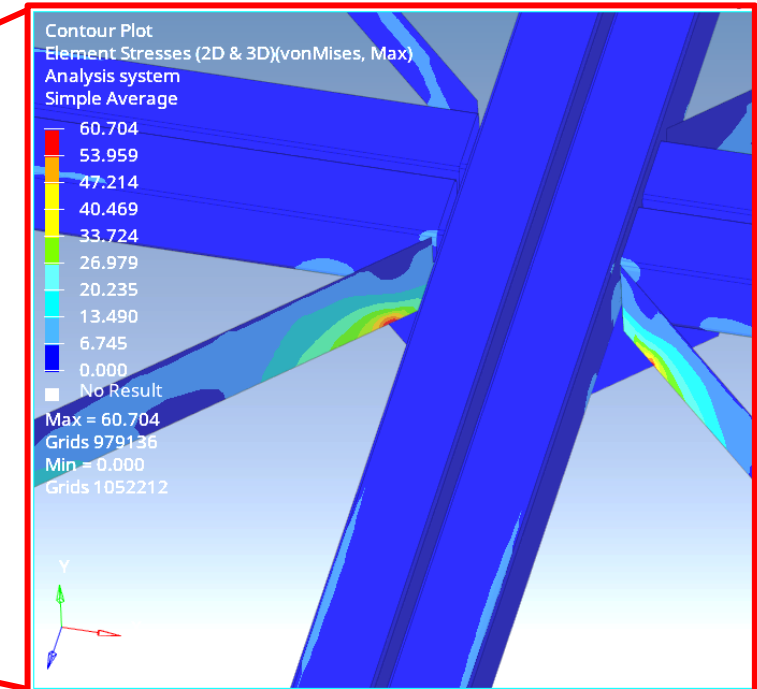
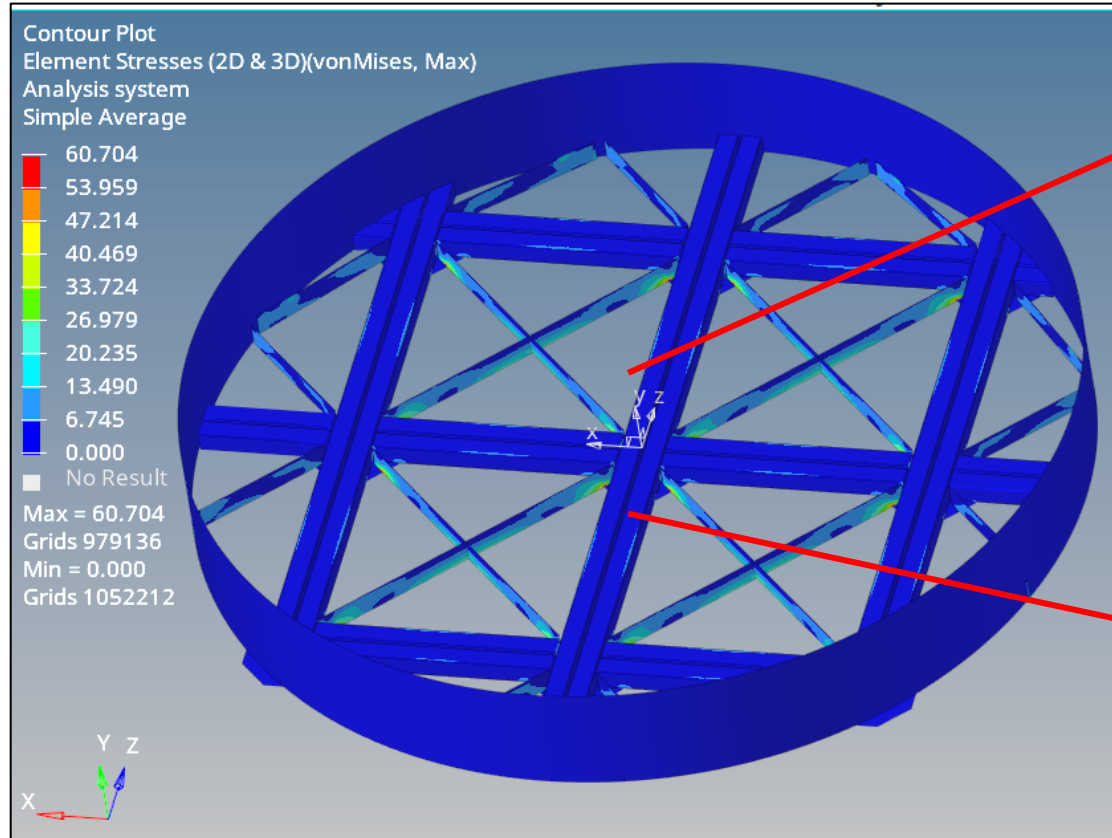
**Max. Displacement = 0.22 mm**



**Max. Stress = 60.70 MPa**



# Base Support Results

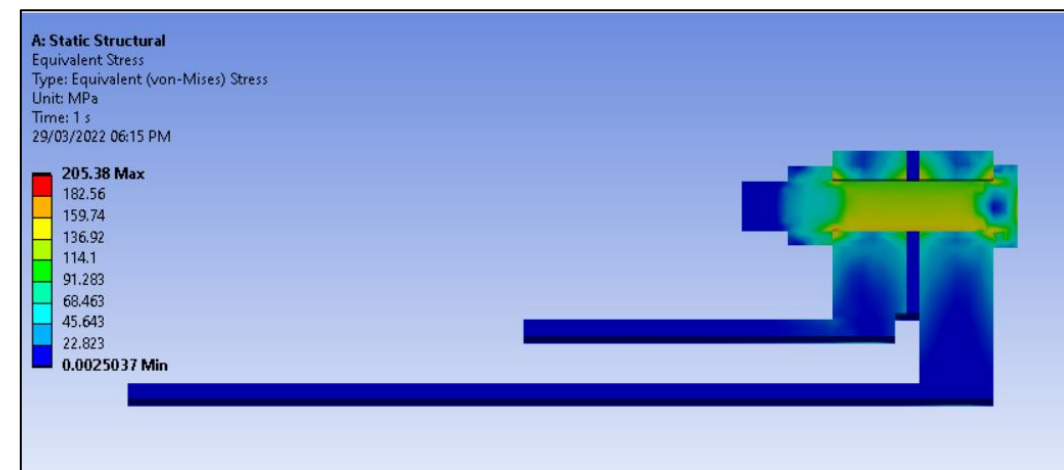
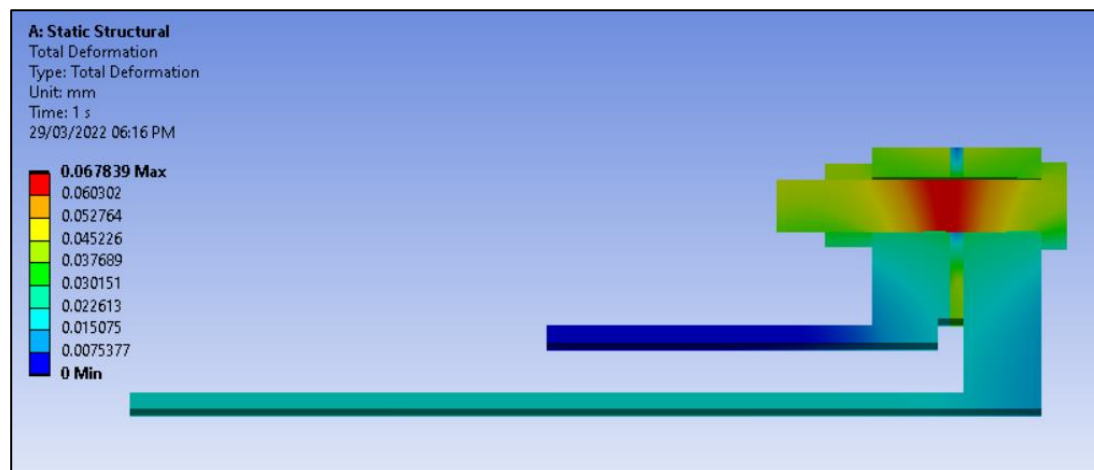
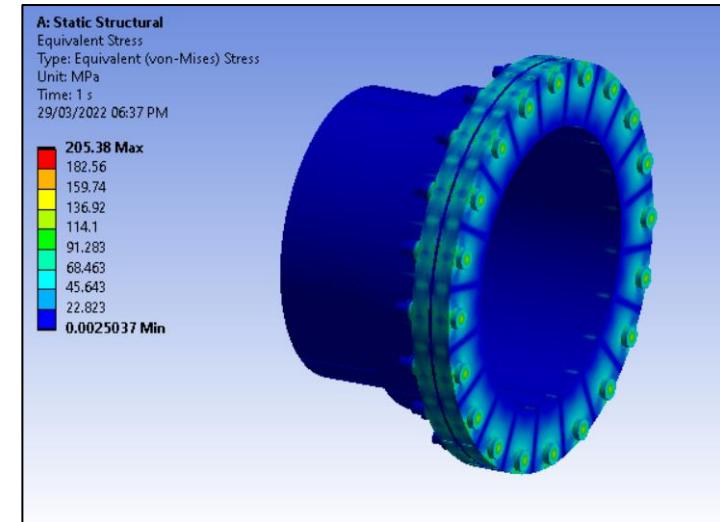


**Max. Stress = 60.70 MPa**

# Tertiary Beam Window Results

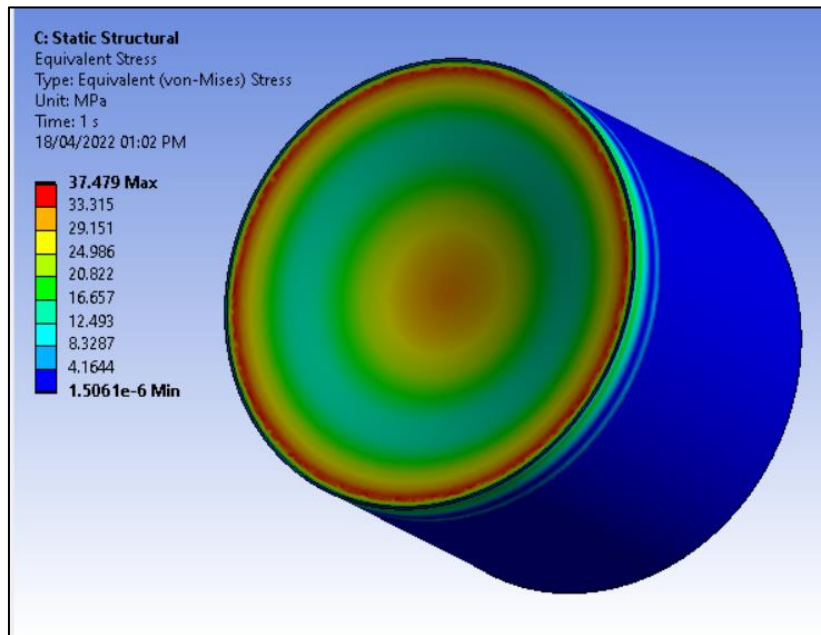
- Overall, the max, stress is in bolt due to preload of about 150 MPa.
- The stress on Flanges is about 50 MPa and on cylinder is 40 MPa.

The Design seems to be safe in this FEA Study.

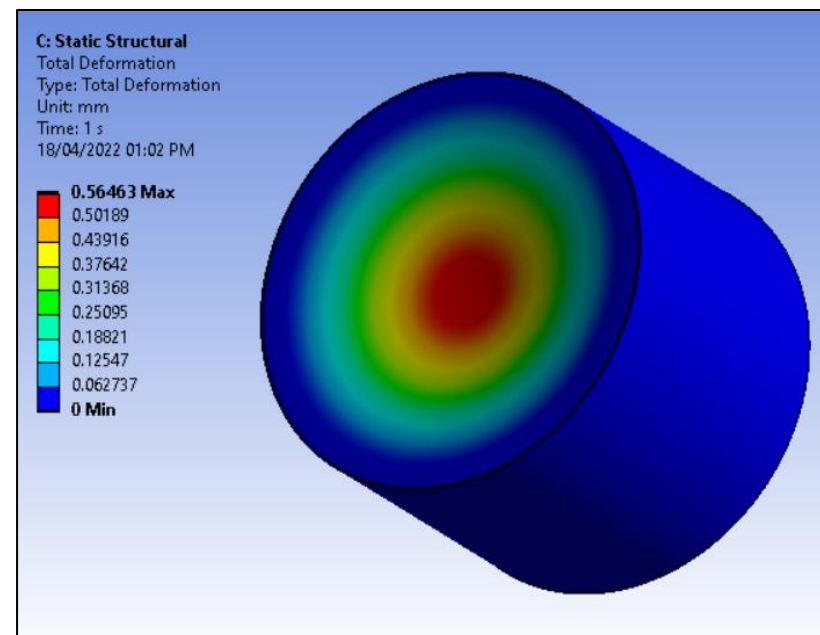




# Tertiary Beam Window Results



**Max. Stress = 38 MPa**



**Max. Displacement = 0.57 mm**

# Conclusion

1. From the results we can infer that
2. The maximum stress and displacement due to Hydrostatic Pressure on the Tank assembly occurs on the Tank base of **60.70 MPa** .
3. The maximum displacement is obtained on the tertiary beam i.e., **0.50mm**
4. In the study, the model appears to be safe by the **factor of safety 3.46** ( w.r.t to yield stress of 207 MPa)

Tank		Tertiary Beam Window		Base Mesh		Factor of Safety
Max. VonMises stress (MPa)	Max. Displacement (mm)	Max. VonMises stress (MPa)	Max. Displacement (mm)	Max. VonMises stress (MPa)	Max. Displacement (mm)	
57.42	0.67	45.20	0.50	60.70	0.22	3.46

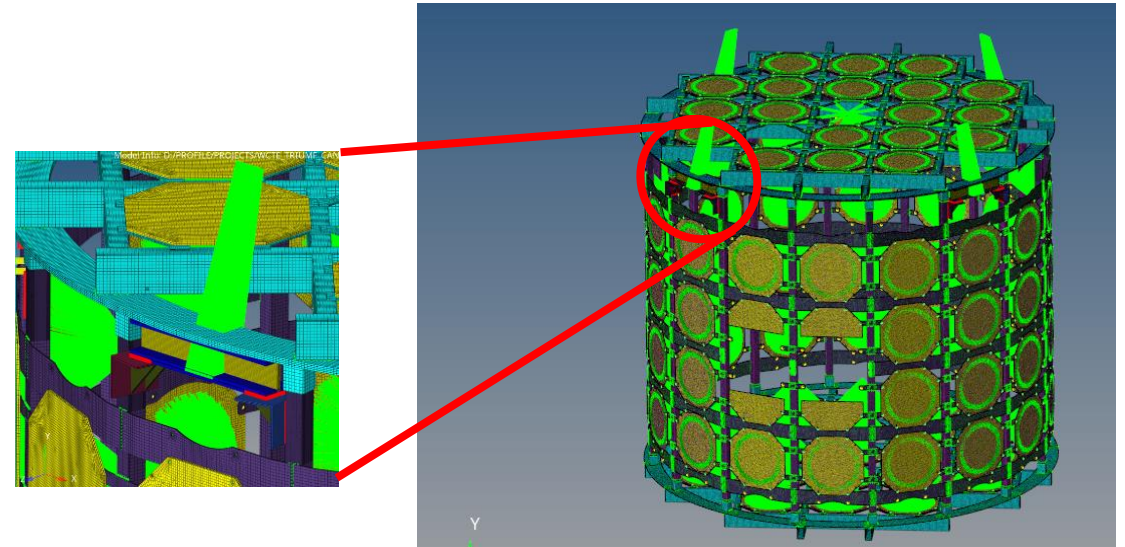
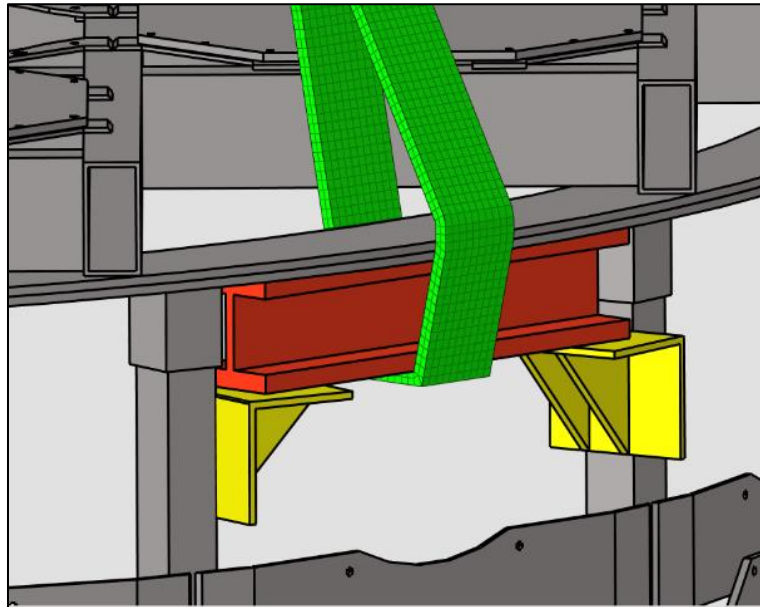
# Support Structure Lifting Analysis

## Boundary Condition

The Bottom End of the Beam Fixed

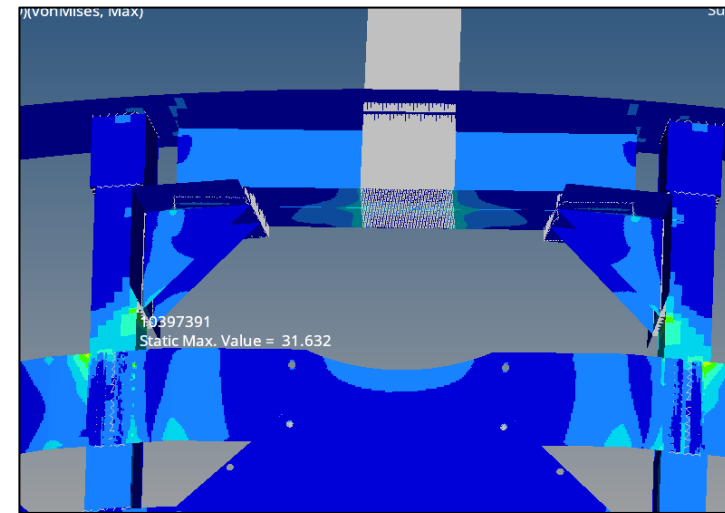
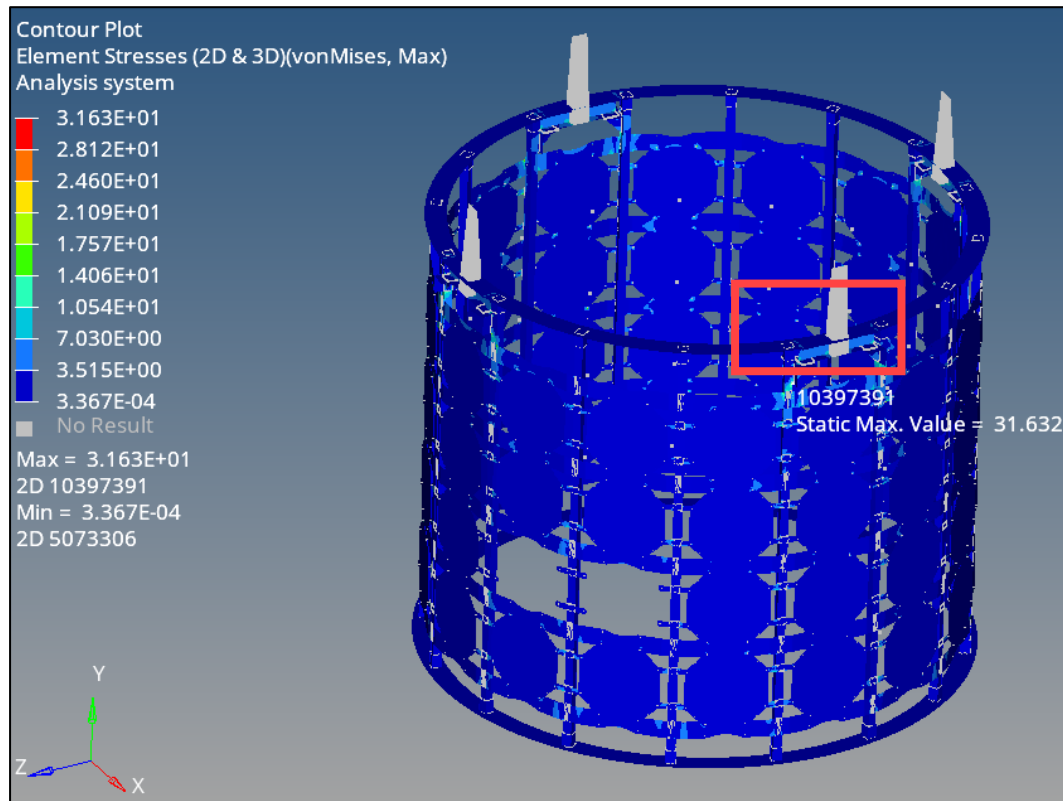
## Loads

Self wt.( Gravity)+ 102 mPMT wt. 40kgs each



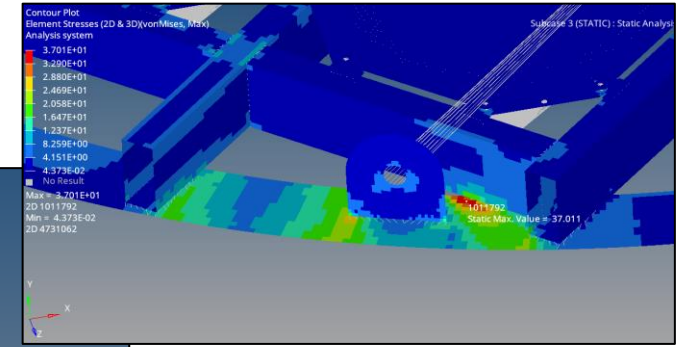
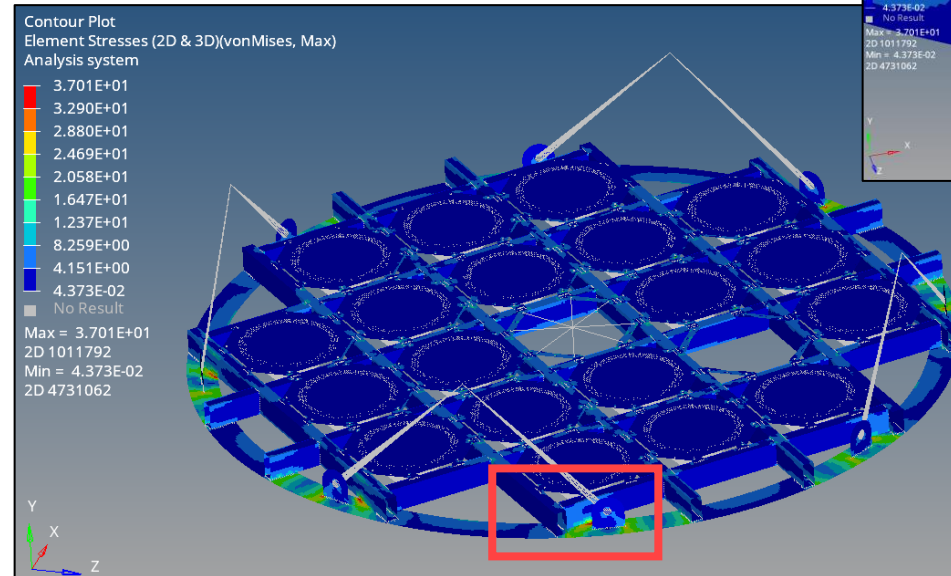
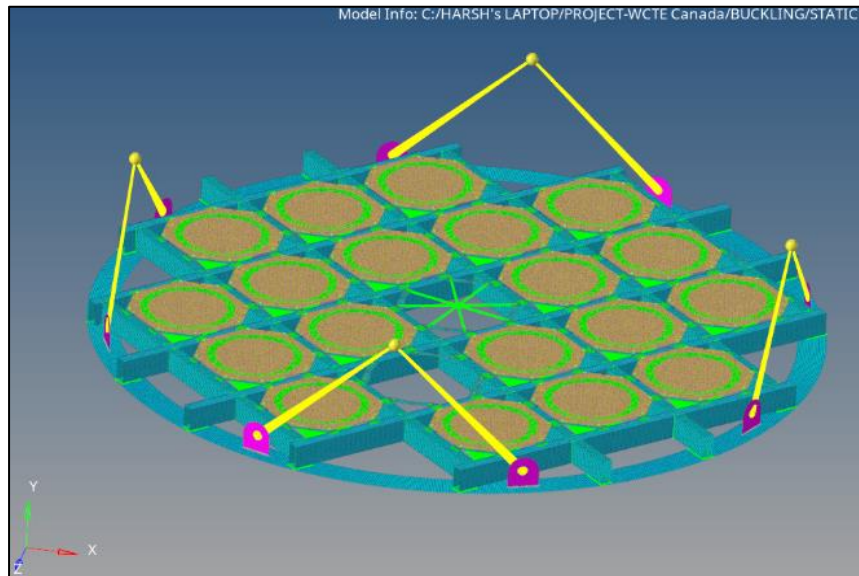
# Support Structure Lifting Analysis

## Support Structure Lifting Results



**Max. Stress = 32 MPa**

# End Cap Lifting Analysis



**Max. Stress = 37 MPa**

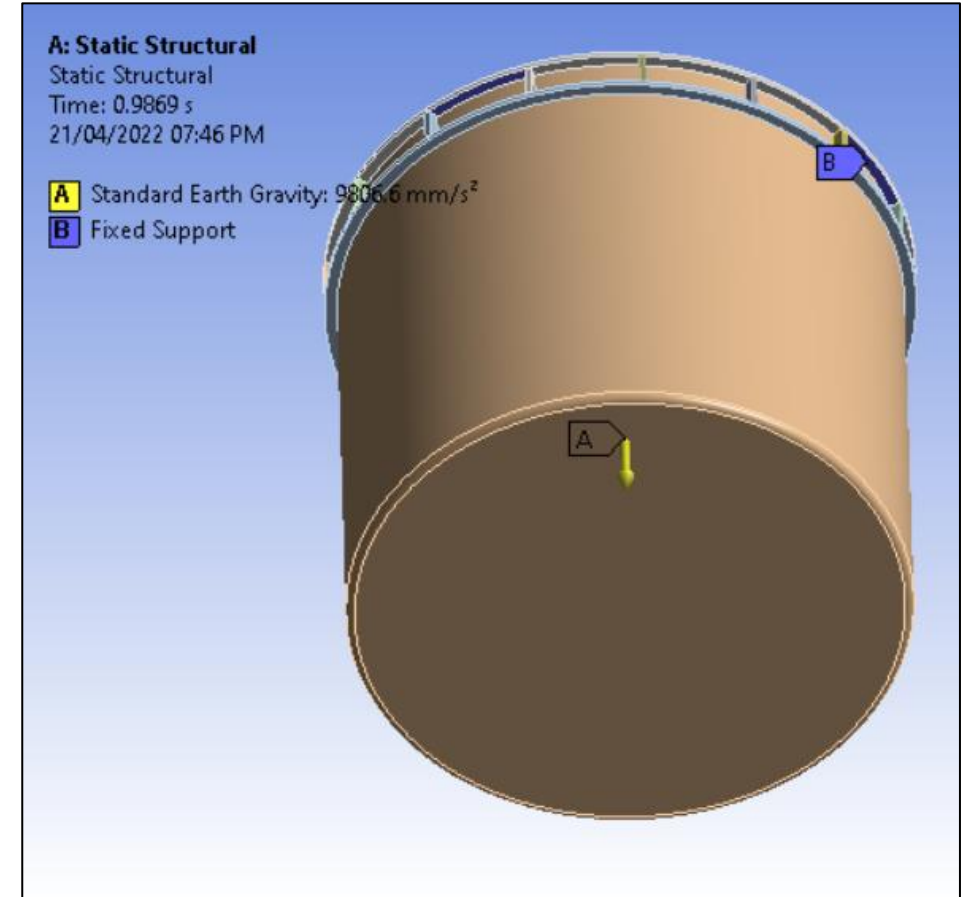
# Tank Lifting Analysis

## Boundary Condition

Top Ring Constrained

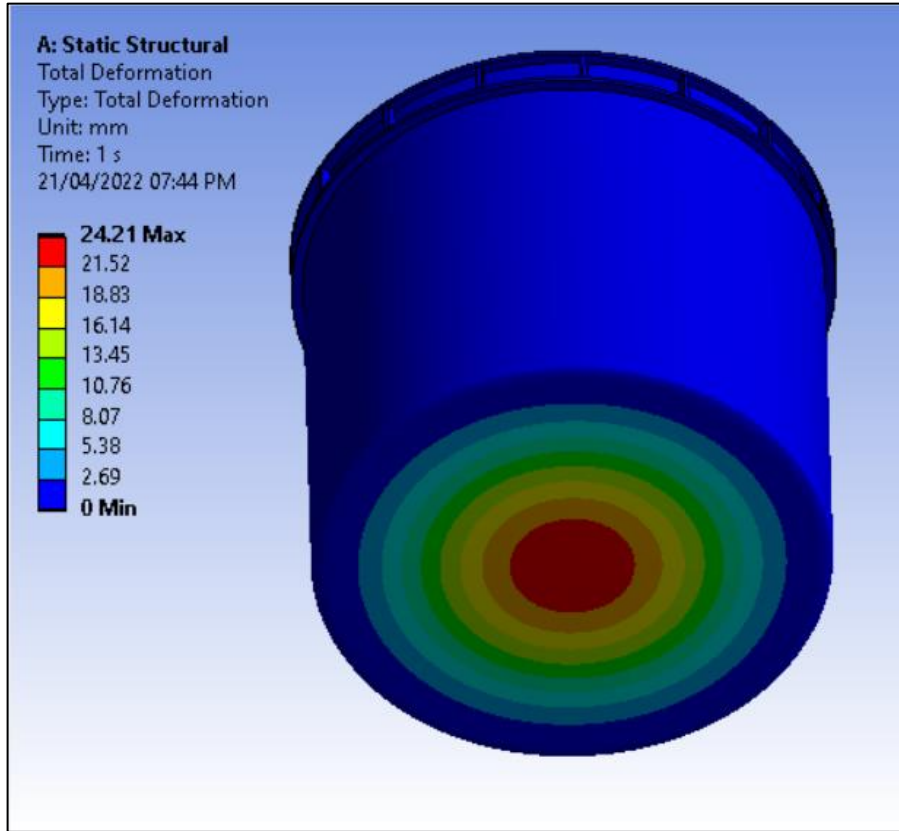
## Loads

Self wt.( Gravity)

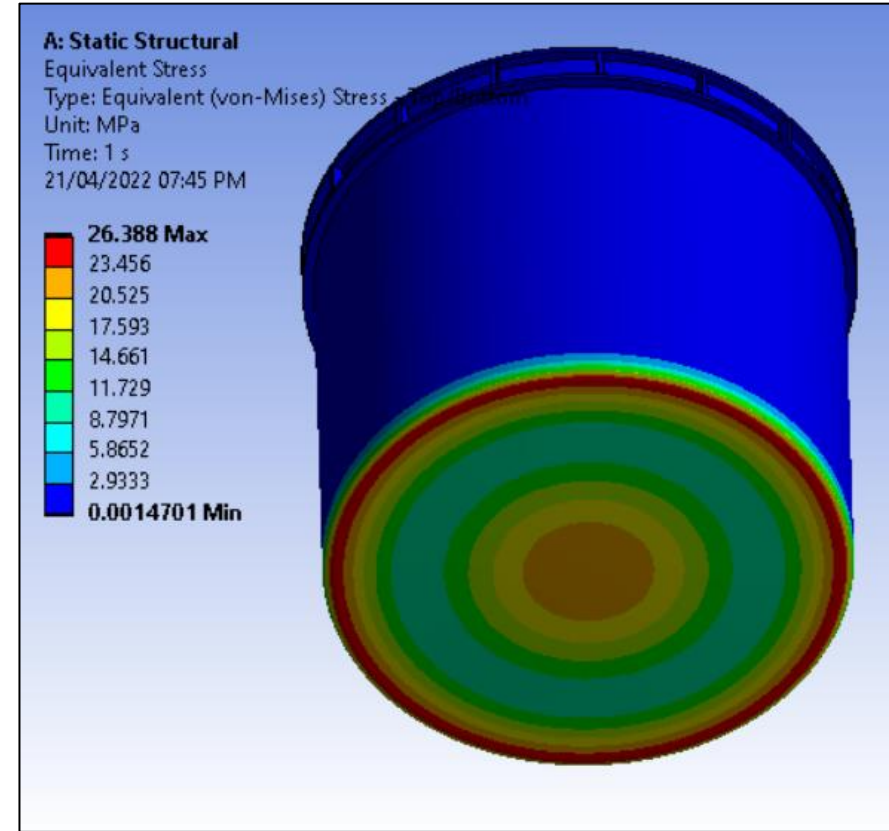




# Tank Lifting Analysis Results



**Max. Displacement = 24 mm**



**Max. Stress = 27 MPa**

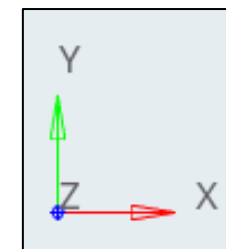
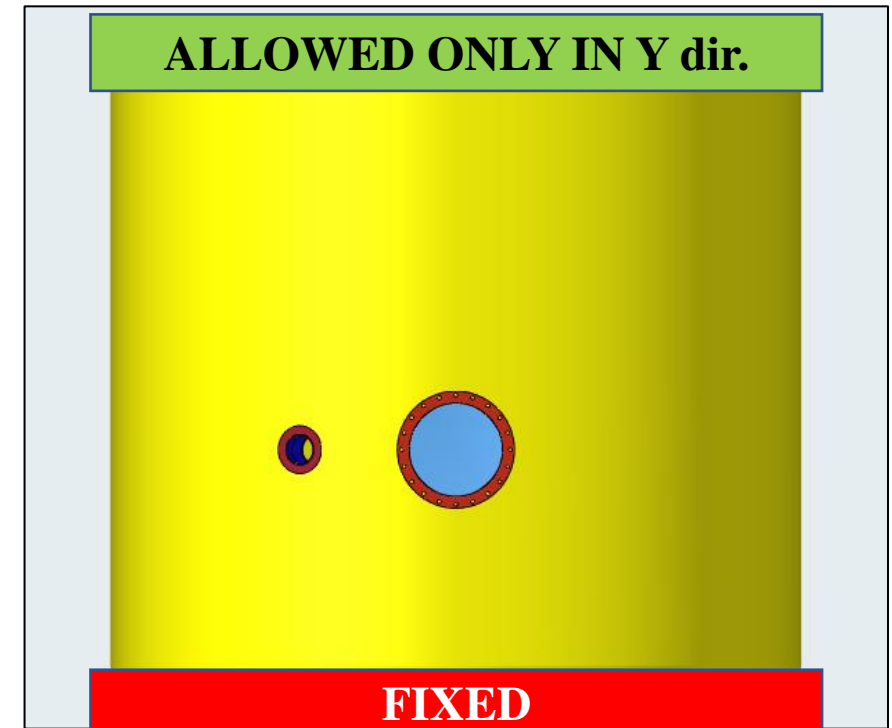
# Buckling Analysis

## Boundary Condition

- The base are restricted to all dofs.
- Top edge is only allowed in axial dir. ( Y-dir.)

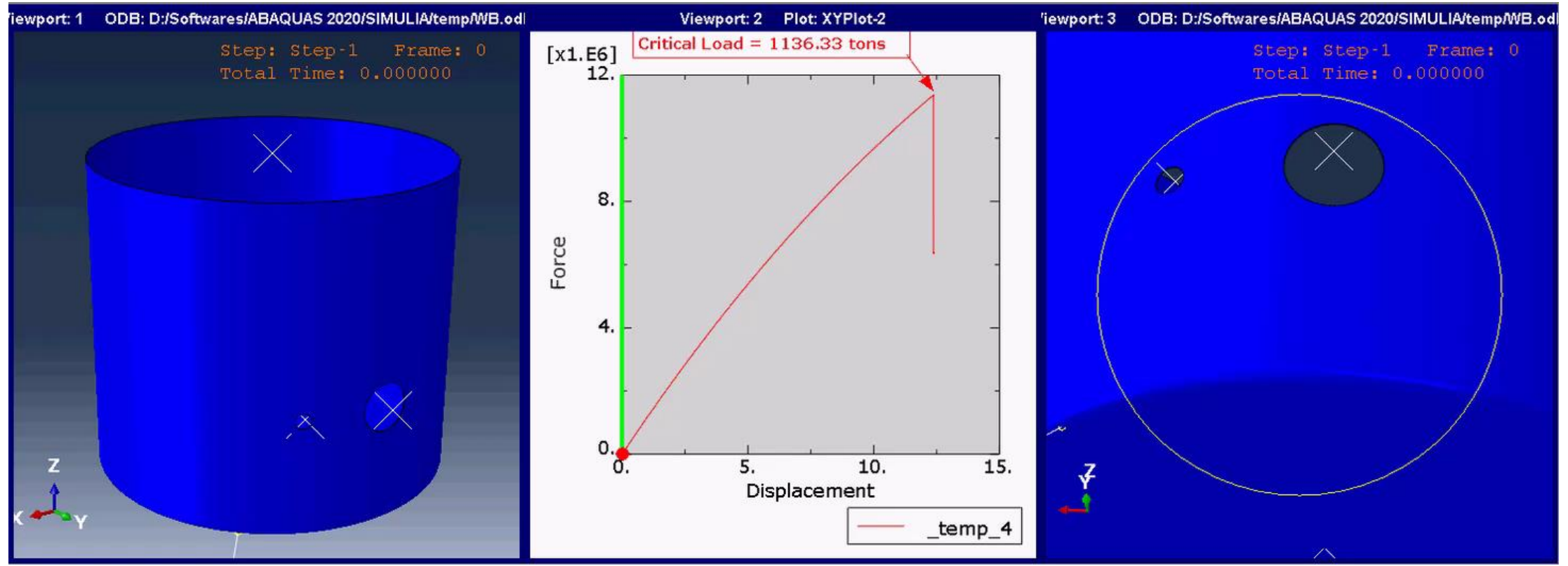
## Tank properties

- Thickness= 6mm
- Length= 3400mm
- Diameter= 3800mm





# Nonlinear Buckling Analysis



Deformation

Force – Displacement response graph

**Non-Linear Buckling Critical Load = 1136 ton. Actual load is 2 tons. Hence Safe**

# Conclusion

- The critical buckling load of the Tank in Non-Linear Buckling analysis under axial compression is **1136 tons** .
- Since the **self weight ( 2 tons) is less than calculated critical buckling load**, the Tank is safe to buckling.

Analysis	Critical Load
Linear Buckling	2633 tons
Nonlinear Buckling	1136 tons

# Modal Analysis

## Boundary Condition

The bottom plates are restricted to all dofs.

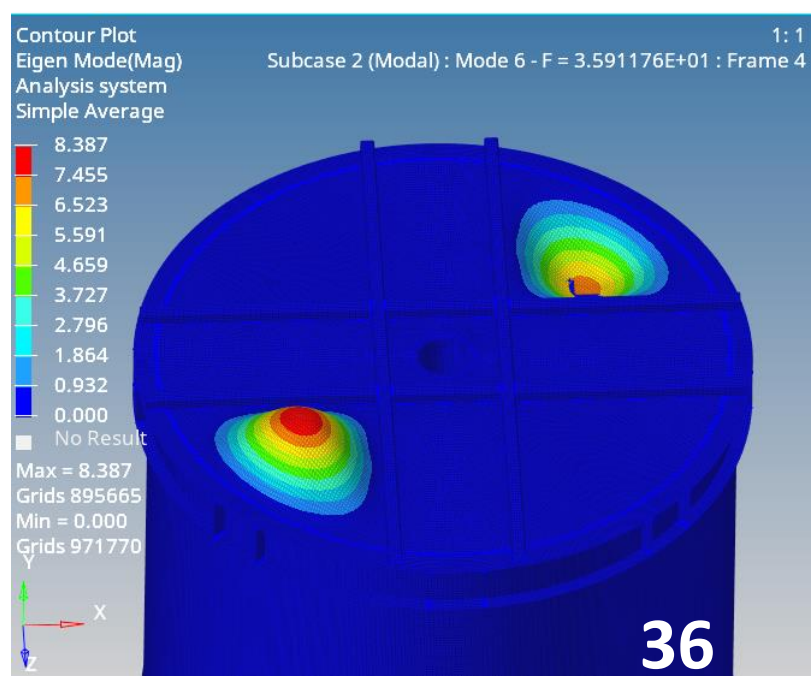
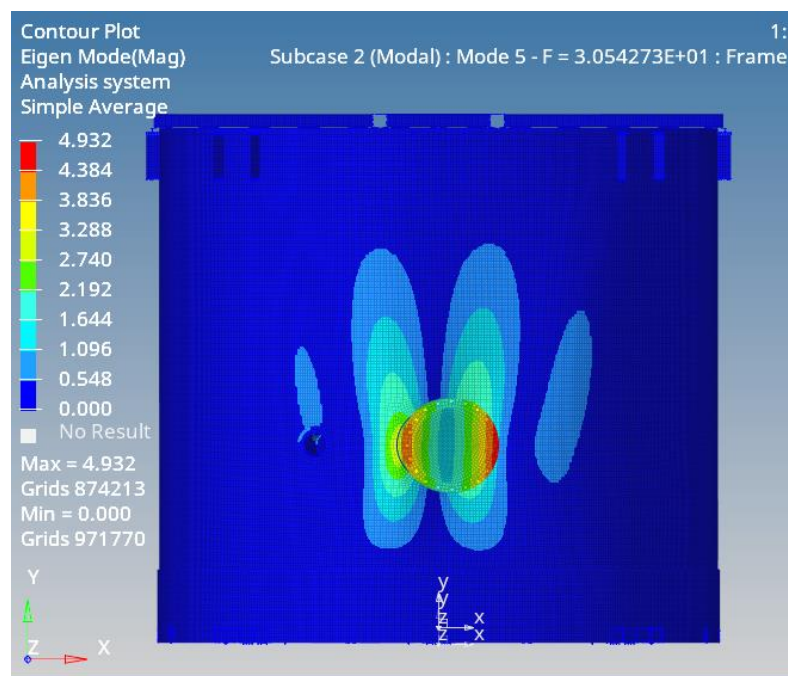
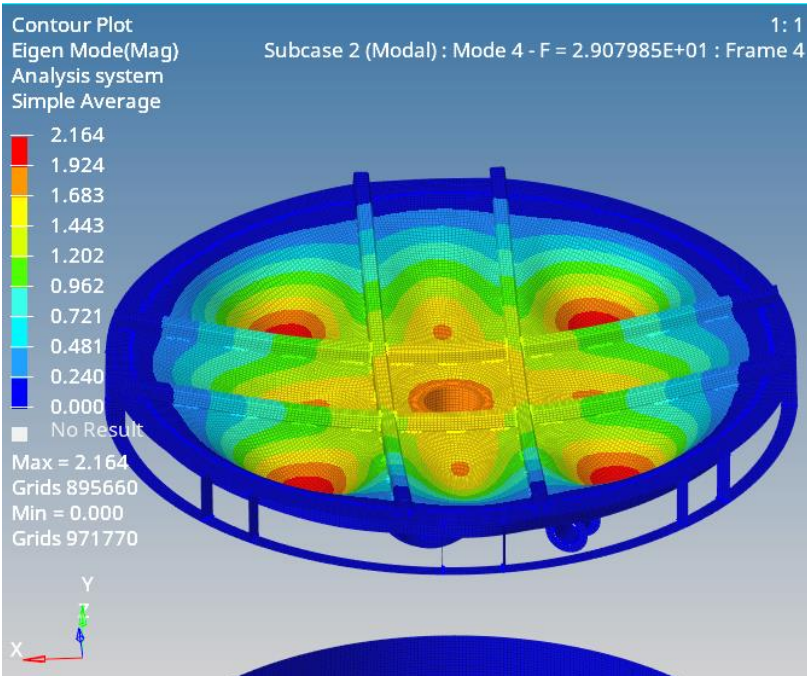
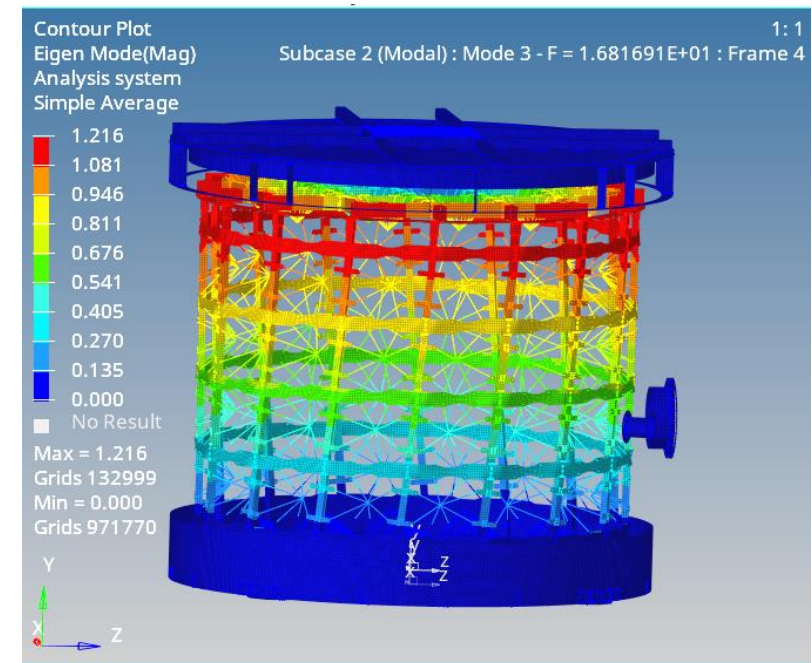
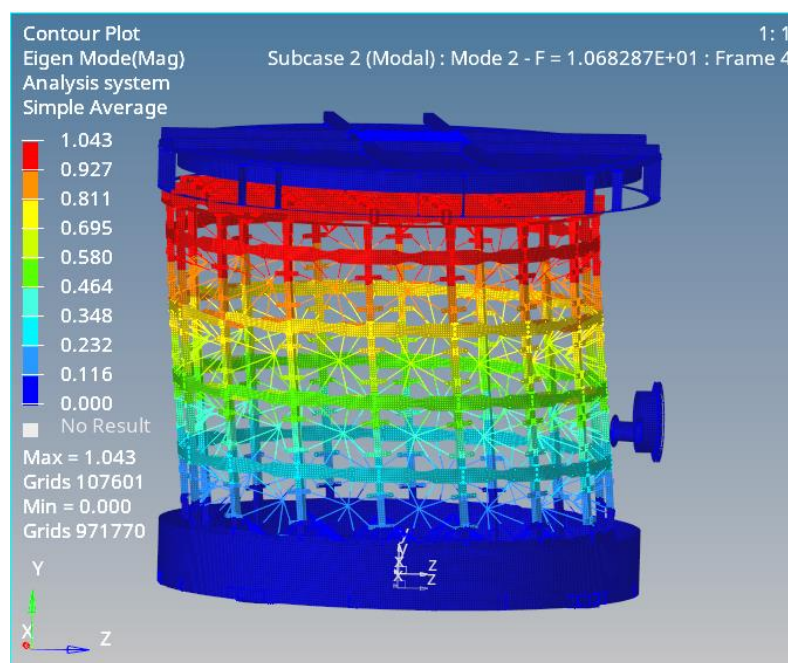
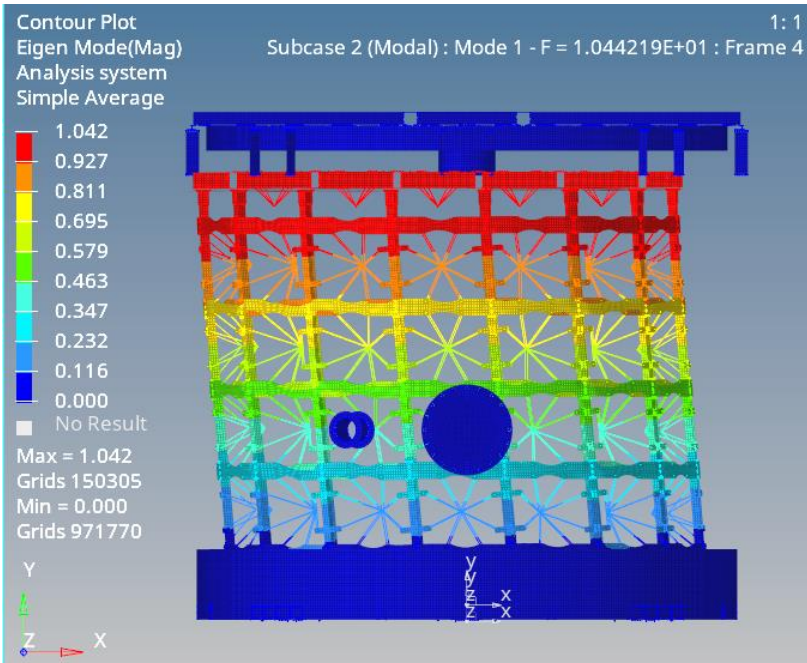
## Loads

Self wt.( Gravity)+ Hydrostatic Pressure on tank walls  
( free surface ht. =3.4m) +100 kg (CDS) on top rafters

Analysis up to 50 Natural Modes

Structure and Tank are not coupled in this Study





## Results

MODAL PARTICIPATION FACTORS FOR SUBCASE 3  
RIGID BODY MODES BASED ON REFERENCE POINT AT ORIGIN OF BASIC COORDINATE SYSTEM

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Mode	Frequency	X-TRANS	Y-TRANS	Z-TRANS	X-ROTAT	Y-ROTAT	Z-ROTAT
1	1.044E+01	-1.077E+00	3.128E-07	5.367E-02	1.464E+02	1.731E-01	2.888E+03
2	1.068E+01	-5.391E-02	9.424E-05	-1.068E+00	-2.878E+03	-1.190E-01	1.463E+02
3	1.682E+01	7.458E-05	2.006E-03	-4.195E-05	-1.119E-01	1.656E+03	-2.466E-01
4	2.872E+01	-9.483E-02	-3.873E-02	3.779E-02	5.492E+01	-2.873E+02	5.444E+01
5	2.908E+01	5.511E-03	-8.250E-01	9.631E-03	2.172E+01	1.516E+01	-3.505E+00
6	3.505E+01	2.656E-02	-1.340E-02	2.890E-01	6.916E+02	1.880E+01	-5.038E+01
7	3.591E+01	-2.413E-02	2.247E-03	4.619E-04	2.521E+02	-1.622E+00	3.417E+02
8	3.607E+01	1.763E-02	3.327E-04	1.722E-03	2.932E+02	-3.510E-01	-2.911E+02
9	3.613E+01	-5.311E-03	-4.411E-03	-3.133E-04	7.264E-01	-8.039E-01	7.323E+01
10	3.669E+01	1.778E-03	-3.551E-03	-3.214E-04	4.105E-01	-1.148E+01	-8.333E-02

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Critical frequency of tank with structure is:

- X dir. = 10.44 Hz
- Y dir. = 29.08 Hz
- Z dir. = 10.68 Hz



Thank You

